CHASE

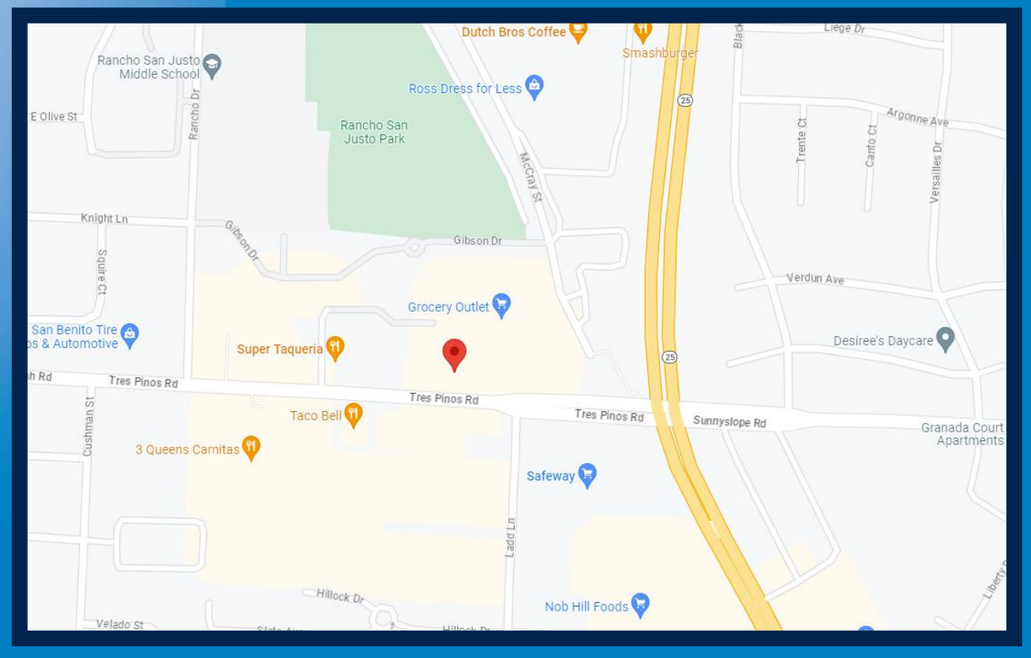
#48600R010443
Hollister
494 Tres Pinos Rd
Hollister, CA 95023

APN:
056-260-001-000

Property Owners Contact:
Tiffani Leibrand -Tiffani.Leibrand@chase.com

Zoning District:
GC-General Commercial

REVISION NOTES:
PERMIT-PYL-R1: Added parcel map and property line and distance form property line for pylon location. - AS - 08/06/25
PERMIT-PYL-R2: Added APN #, Property Owner, Zoning District information on cover page per PM. 9/2/25-AJK



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MEMBER OF CESA CALIFORNIA ELECTRIC SIGN ASSOCIATION

JP Morgan Chase Bank
#48600R010443

Hollister
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Initial Date: 08/11/23
Salesperson: Arthur Navarro
Coordinator: Fabian Marquez
Designer: ASena
Scale: As noted

CUSTOMER APPROVAL

Customer Signature Date

COPY, COLORS & SIZES

Signtech does NOT provide primary electrical to sign location - ***RESPONSIBILITY OF OTHERS!***

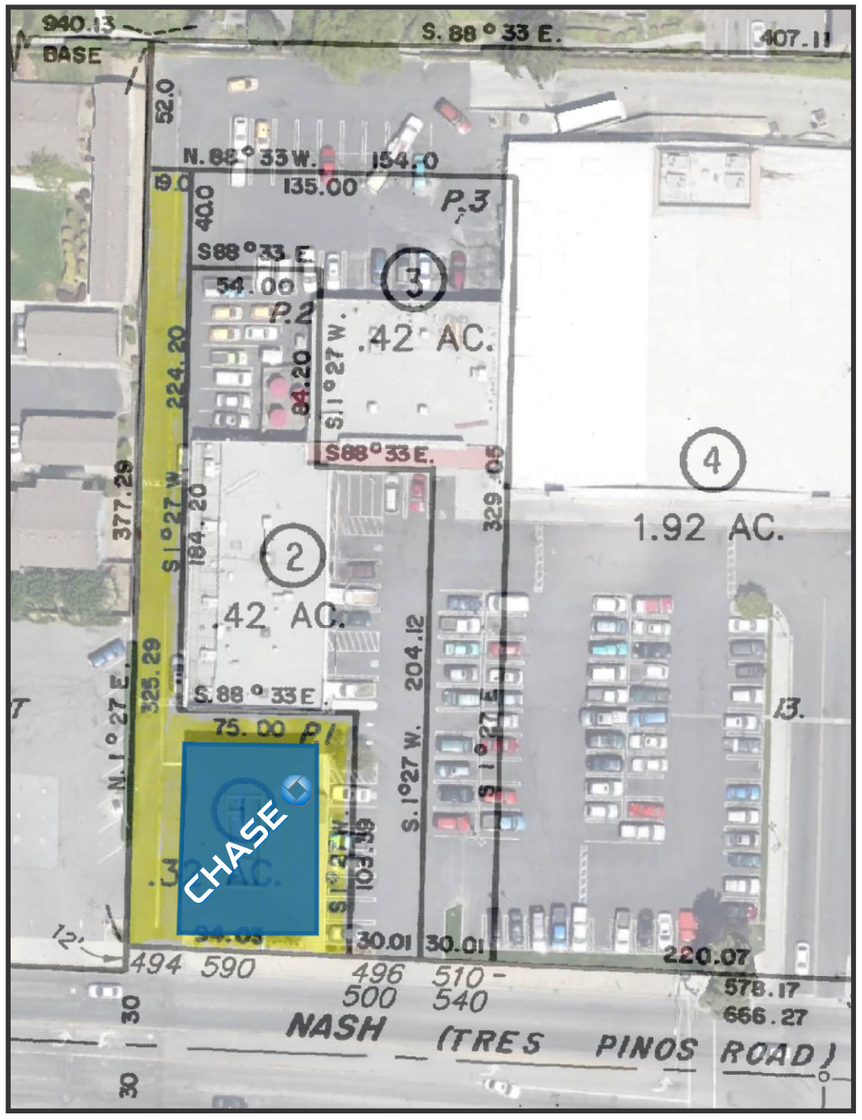
Customer Signature Date

Drawing Number: 23-01425
Project ID: CHASE_48600R010443_1
Revision: PERMIT-PYL-R2: AJK - 09/02/25

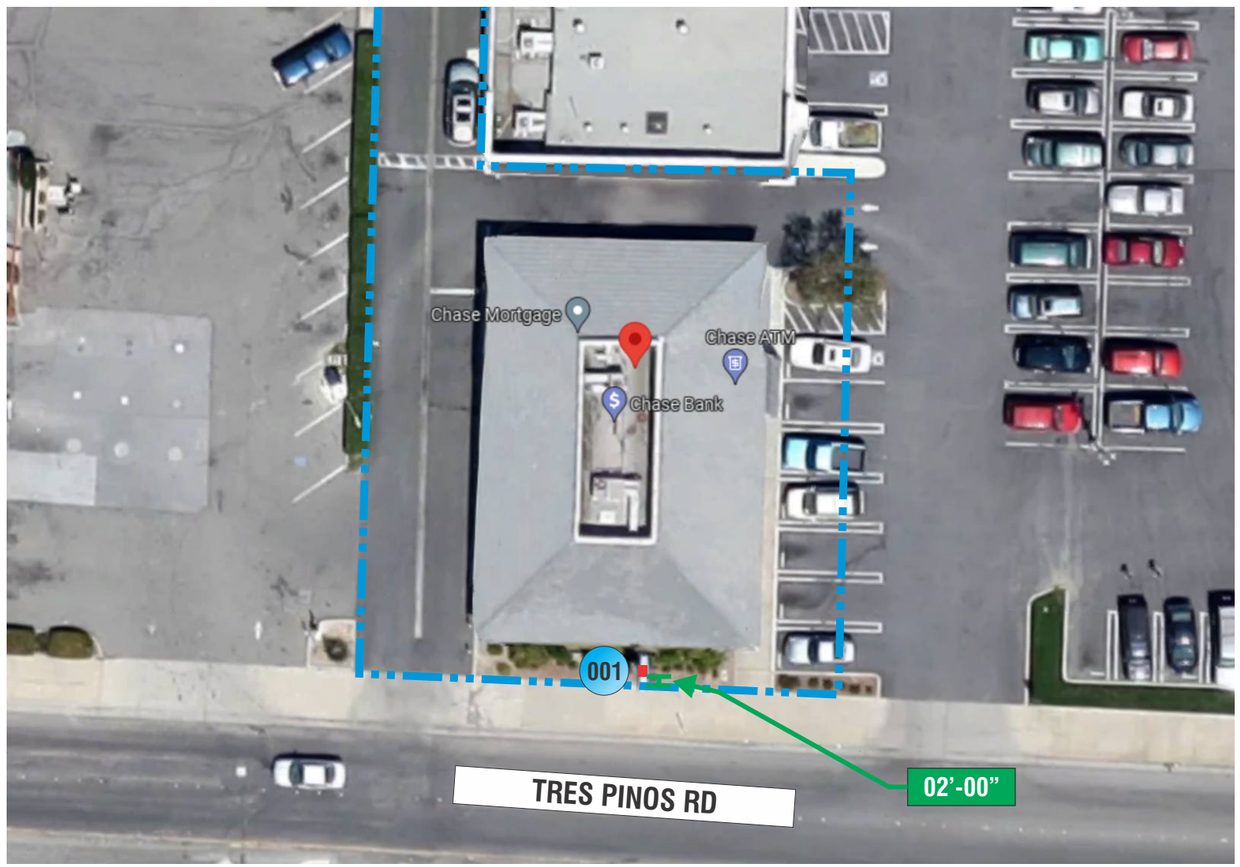
EXTERIOR SIGN LEGEND

Elevation	Sign No.	Sign Type	Description	Sq.Ft.
	001	CUSTOM P-25	Double-Faced Illuminated Pylon - 12 Foot Max Height	25

SQUARE FOOTAGE	
PROPOSED SQFT:	25



PARCEL MAP - SCALE: 1/64”=1’-0”



SITE PLAN SCALE: 1/32”=1’-0”

PROPERTY LINE

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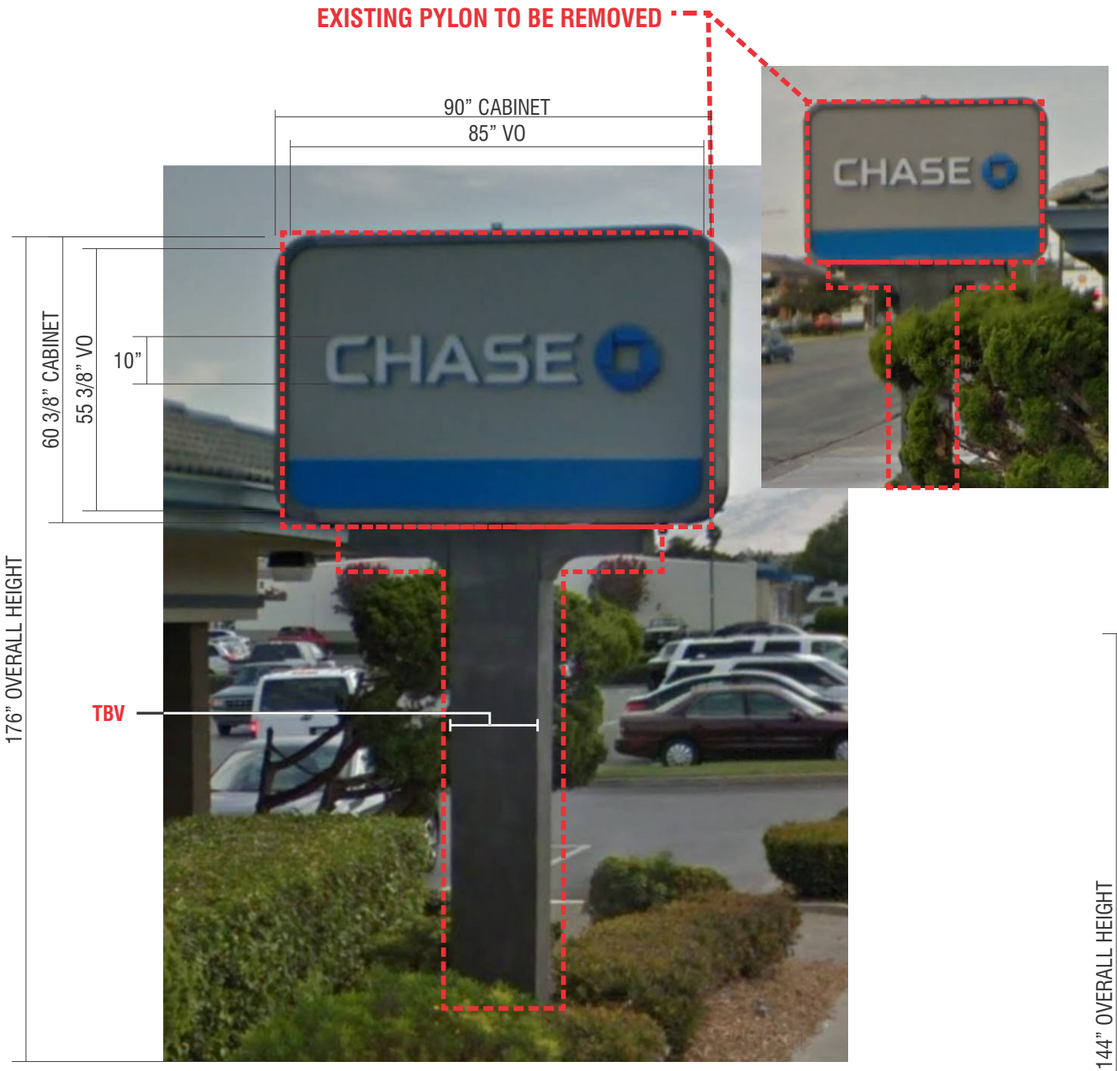
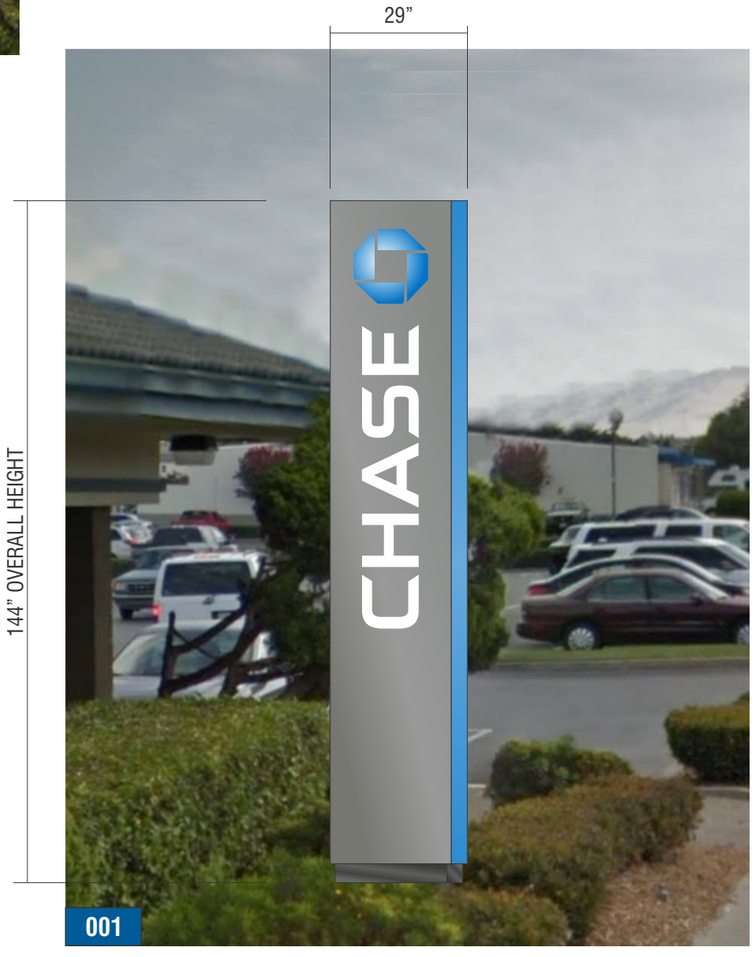
Sign Legend / Site Plan

EXISTING PYLON TO BE REMOVED



EXISTING CONDITIONS SCALE: 3/8”=1’-0”



PROPOSED CONDITIONS SCALE: 3/8”=1’-0”

BACK SIDE

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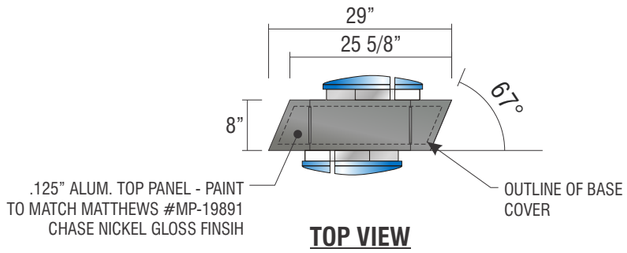
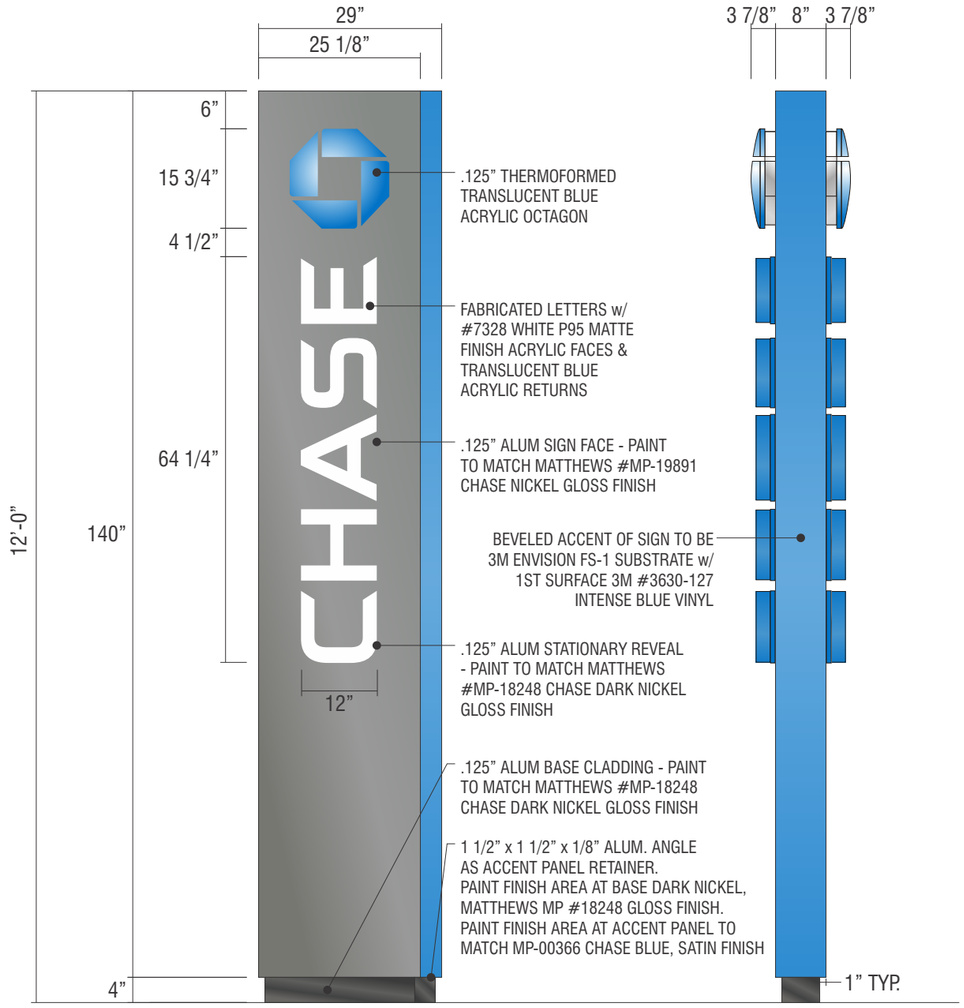


TOP VIEW

ALL EXPOSED PAINTED SURFACES SHALL BE COATED WITH MATTHEWS @282-208SP VOC GLOSS CLEAR, WITH MINIMUM 2 MILS DRY FILM THICKNESS (DFT) PER MATTHEWS APPLICATION SPECIFICATIONS.



SCALE: 1/2”=1’-0”

General Notes:

1. Design is based on a 90/115 mph, 3 second gust wind design per IBC 2009/2012, Category II, Exposure C.
2. Spread foundation is based on a presumptive safe vertical soil bearing pressure minimum of 2000 psf. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact JP MORGAN CHASE.
4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.
5. Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
6. Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
7. All voids between column baseplate and foundation surface shall be completely filled with high-strength, non-shrink grout.
8. Anchor bolts shall meet ASTM F1554 Grade 36. Exposed surfaces shall be galvanized or coated to prevent corrosion.
9. All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a minimum yield strength of 46000 psi. Steel angle, channel and plate shall meet ASTM A36. Extruded aluminum shapes shall be 6061-T6 alloy. Aluminum sheet shall be 3003-H14 alloy. Aluminum plate shall be 5052-H34 alloy.
10. Welds shall be made with E70xx electrodes for steel & with a 4000 series filler for aluminum by persons qualified in accordance with AWS standards within the past two years.
11. All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Tighten all high strength bolts using the Turn-of-Nut method unless noted otherwise.
12. JP Morgan Chase will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
13. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from JP Morgan Chase voids this drawing in its entirety.
14. This design is prototypical and should not be used for site specific applications unless reviewed and deemed suitible for use at that site by a competent Professional Engineer.

ALL EXPOSED FASTENER HEADS SHALL BE PAINTED TO MATCH THE EXTERIOR CABINET FINISH

GRAPHICS DEPICTED ARE FOR ILLUSTRATIVE PURPOSES ONLY! USE ONLY APPROVED ARTWORK FOR PRODUCTION.

001

SIGN TYPE **CUSTOM P-25** **- 12 FOOT MAX HEIGHT**

MANUFACTURE AND INSTALL ONE (1) INTERNALLY ILLUMINATED D/F PYLON SIGN

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CUSTOMER APPROVAL

Customer Signature Date

COPY, COLORS & SIZES

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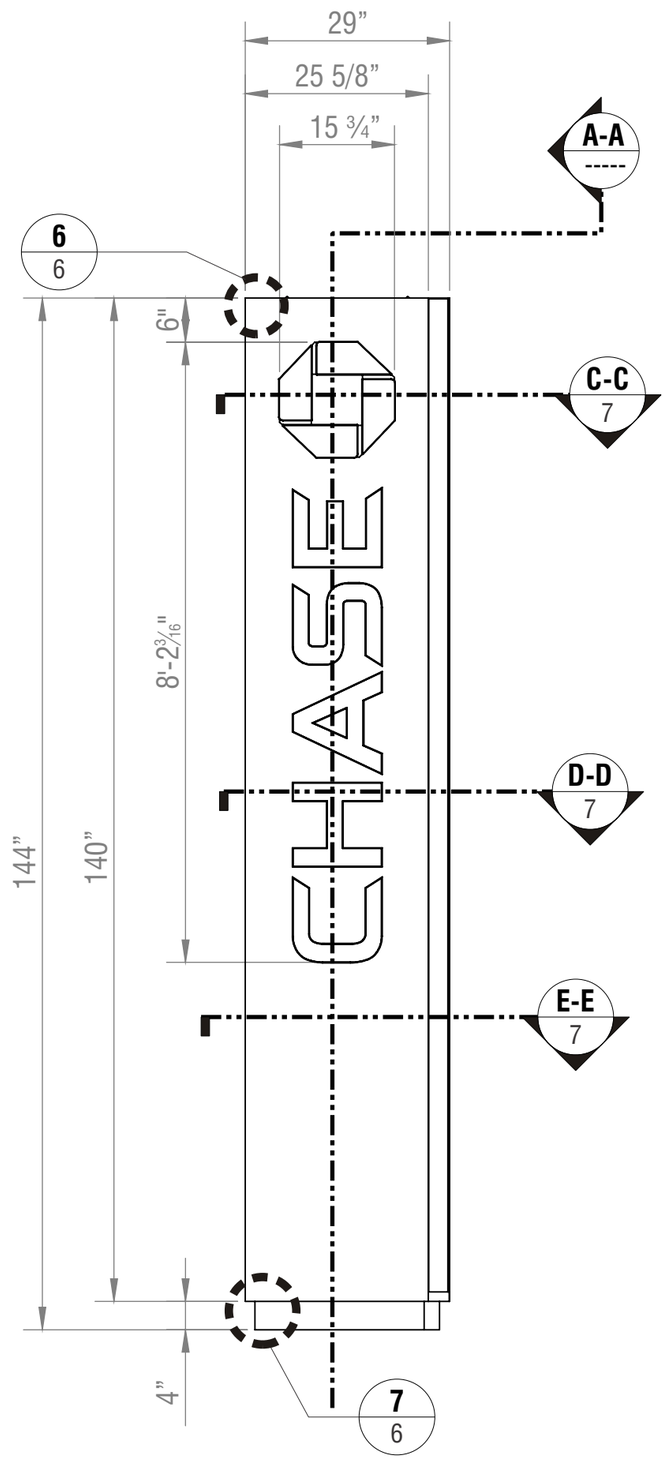
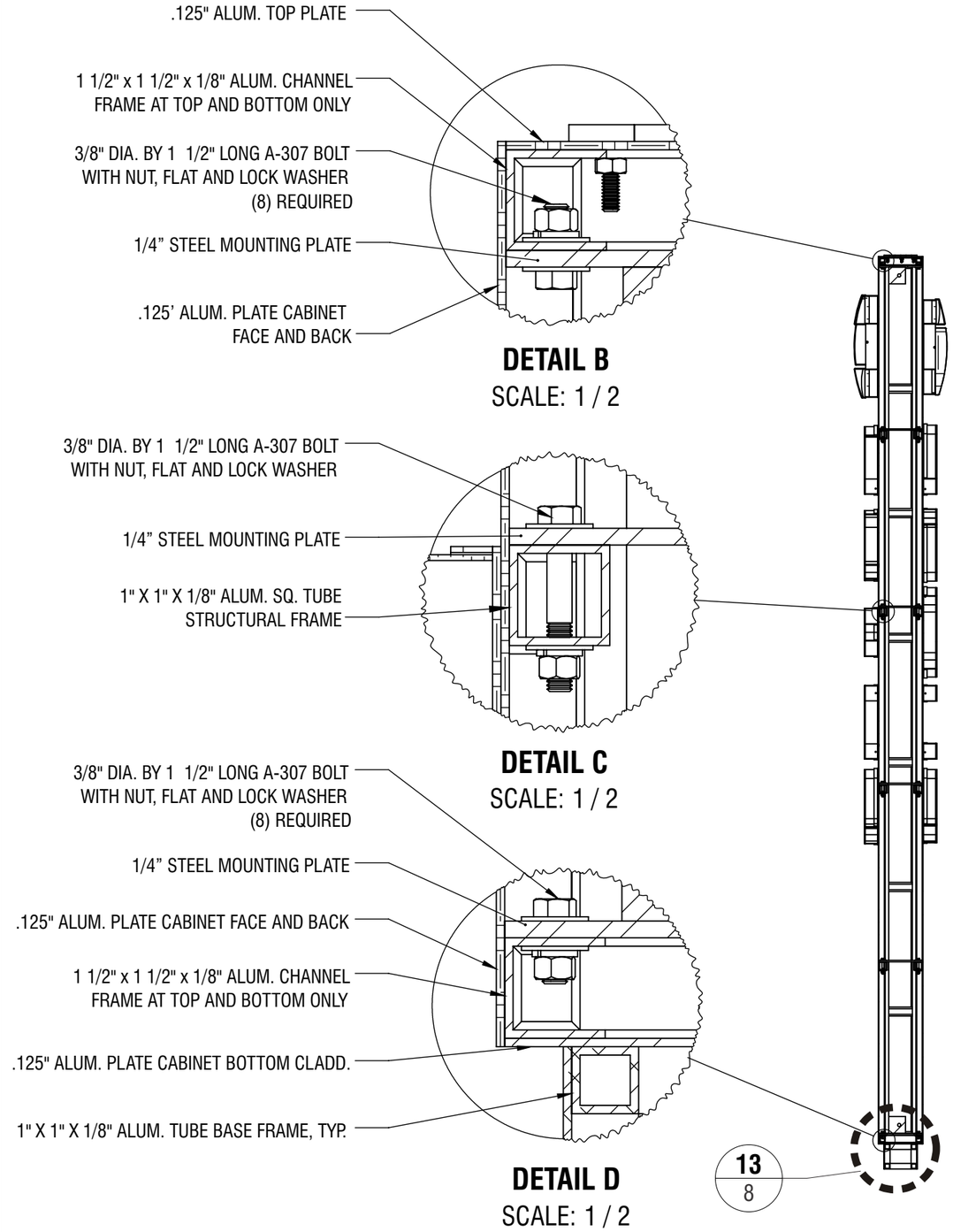


5 | CABINET FRONT ELEVATION | Scale: 1/2” = 1’-0”



A-A | VERTICAL SECTION | Scale: 1/2” = 1’-0”

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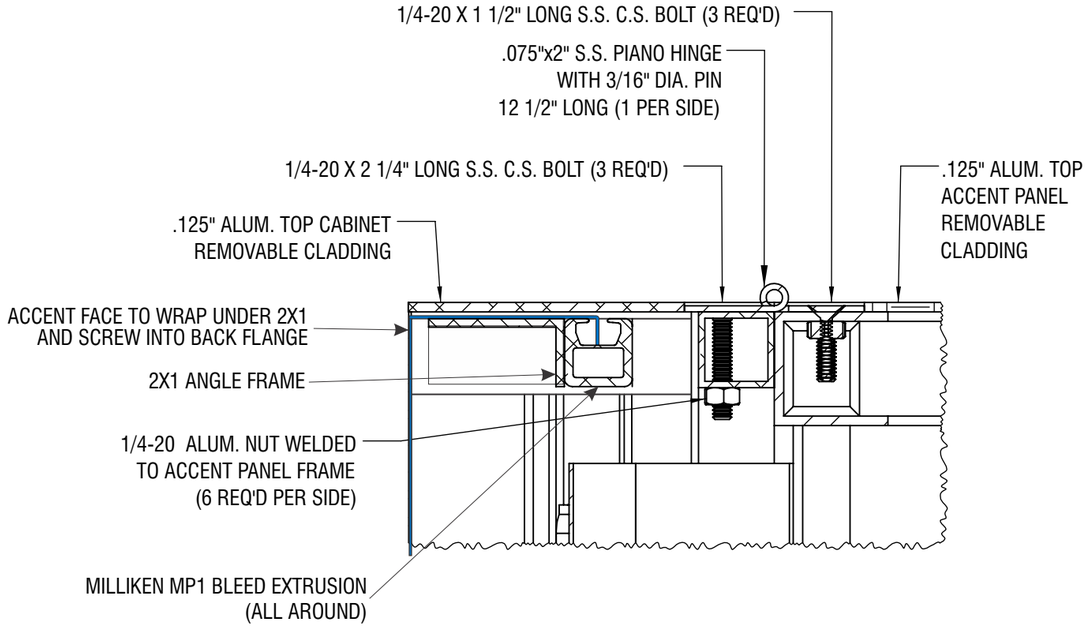
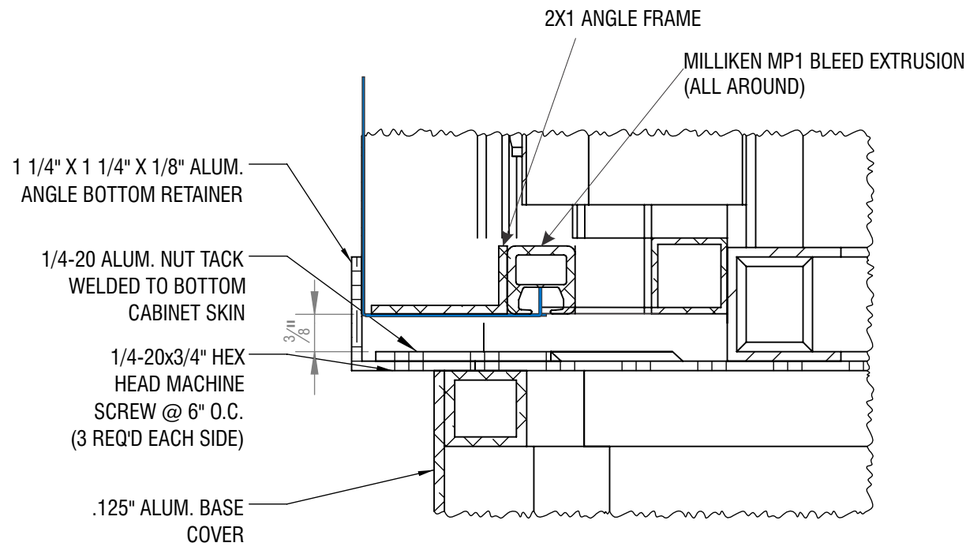


6 HINGE ATTACHMENT DETAIL

Scale: 6” = 1’-0”



7 BOTTOM RETAINER ATTACHMENT DETAIL

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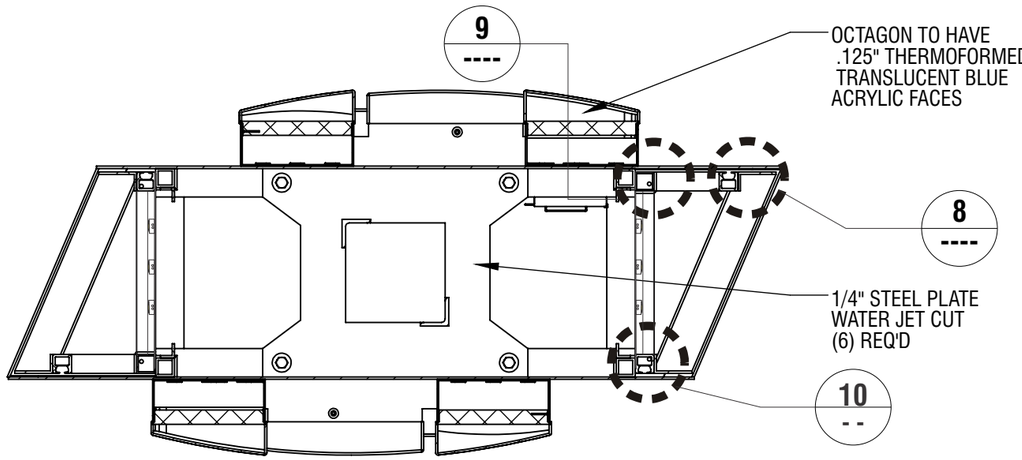
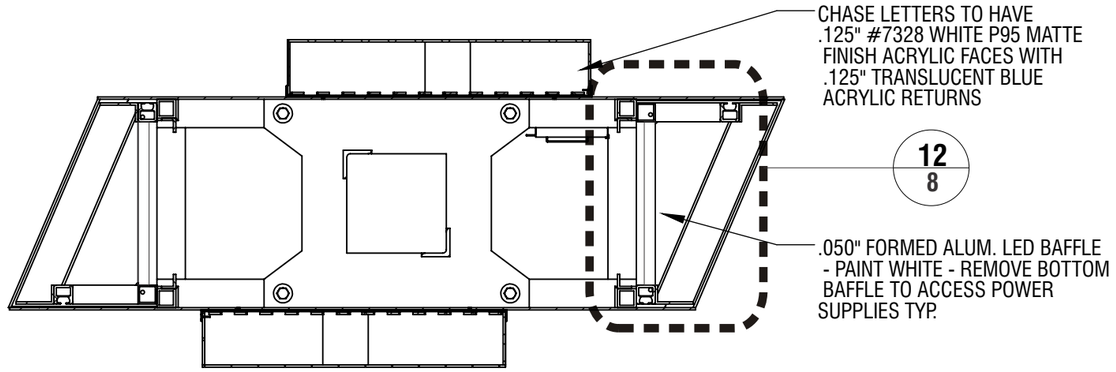
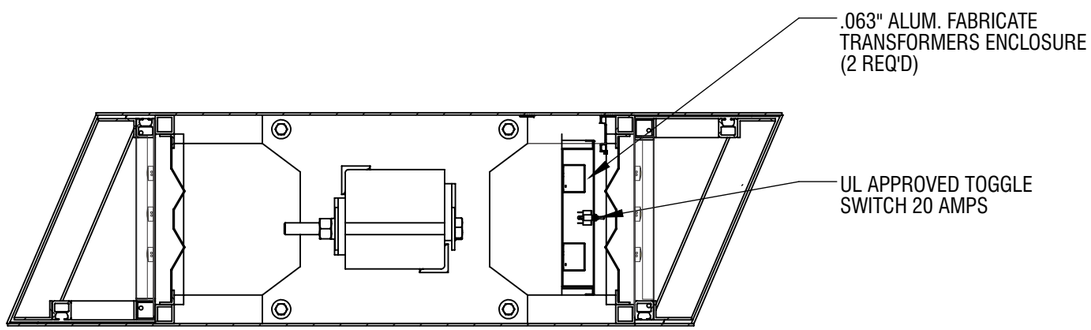


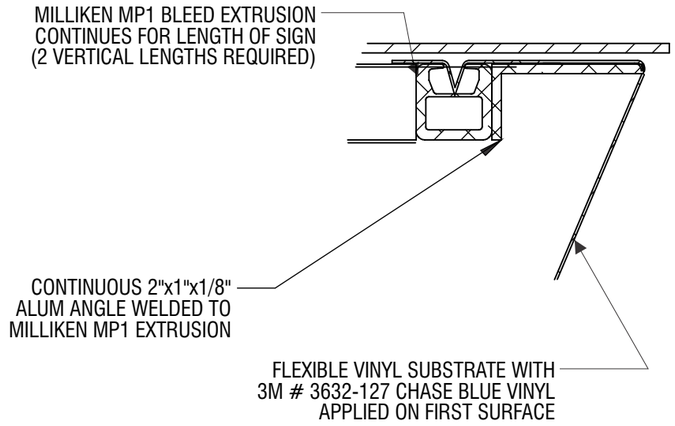
C-C T-T HORIZONTAL SECTION @ OCTAGON — Scale: 1” = 1’-0”



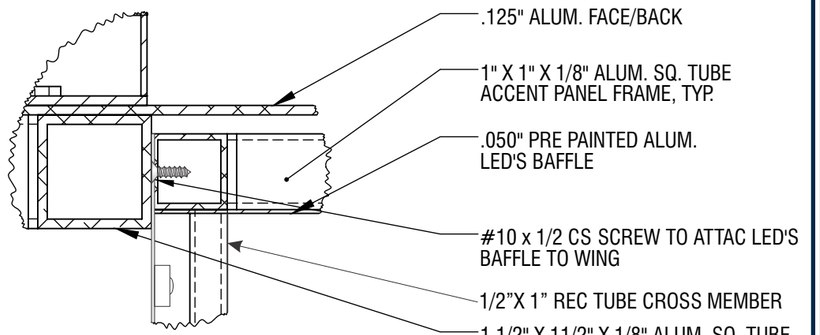
D-D S-S HORIZONTAL SECTION @ LETTERS — Scale: 1” = 1’-0”



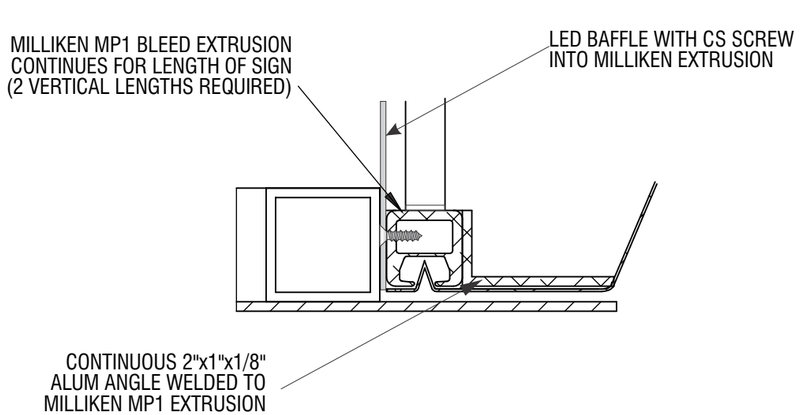
E-E R-R HORIZONTAL SECTION @ TRANSFORMERS — Scale: 1” = 1’-0”



8 DETAIL — Scale: 6” = 1’-0”



9 — Scale: 6” = 1’-0”



10 DETAIL — Scale: 6” = 1’-0”

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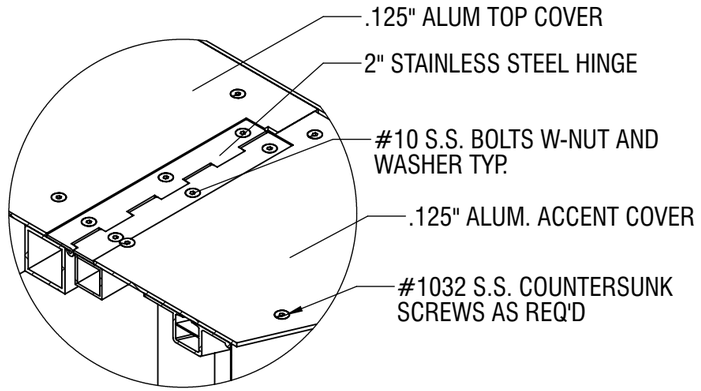
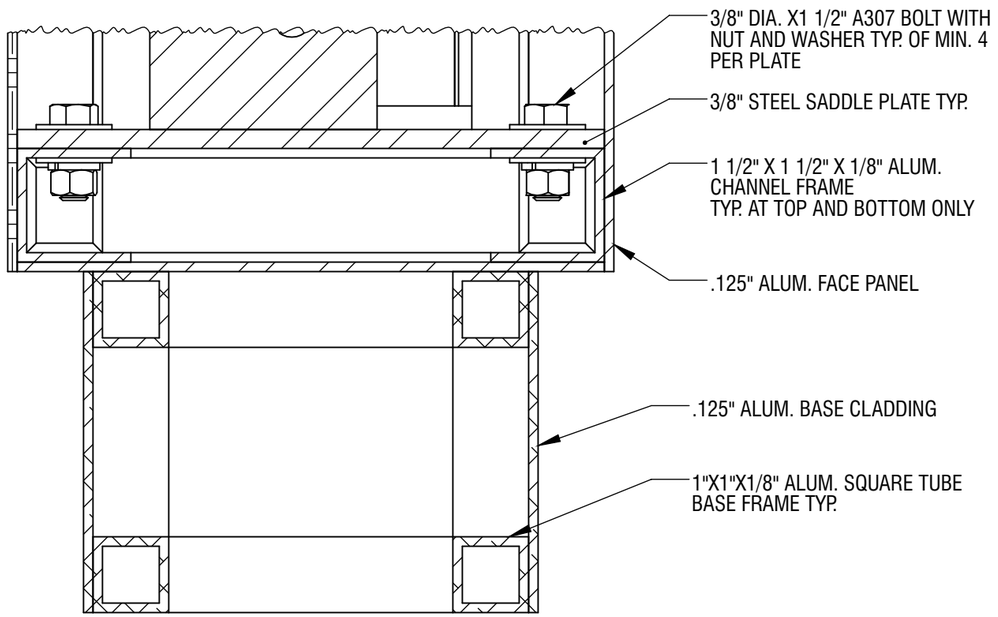


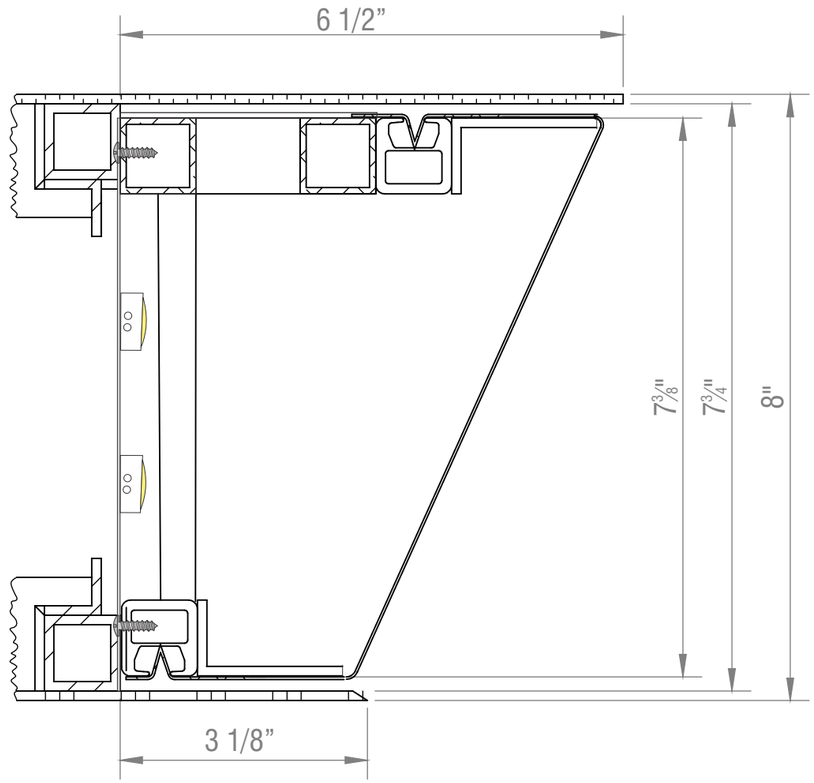
14 HINGE DETAIL ISOMETRIC VIEW

Scale: 3” = 1’-0”



13 BASE TO CABINET ATTACHMENT DETAIL

Scale: 6” = 1’-0”



12 ACCENT PANEL TO CABINET ATTACHMENT DETAIL

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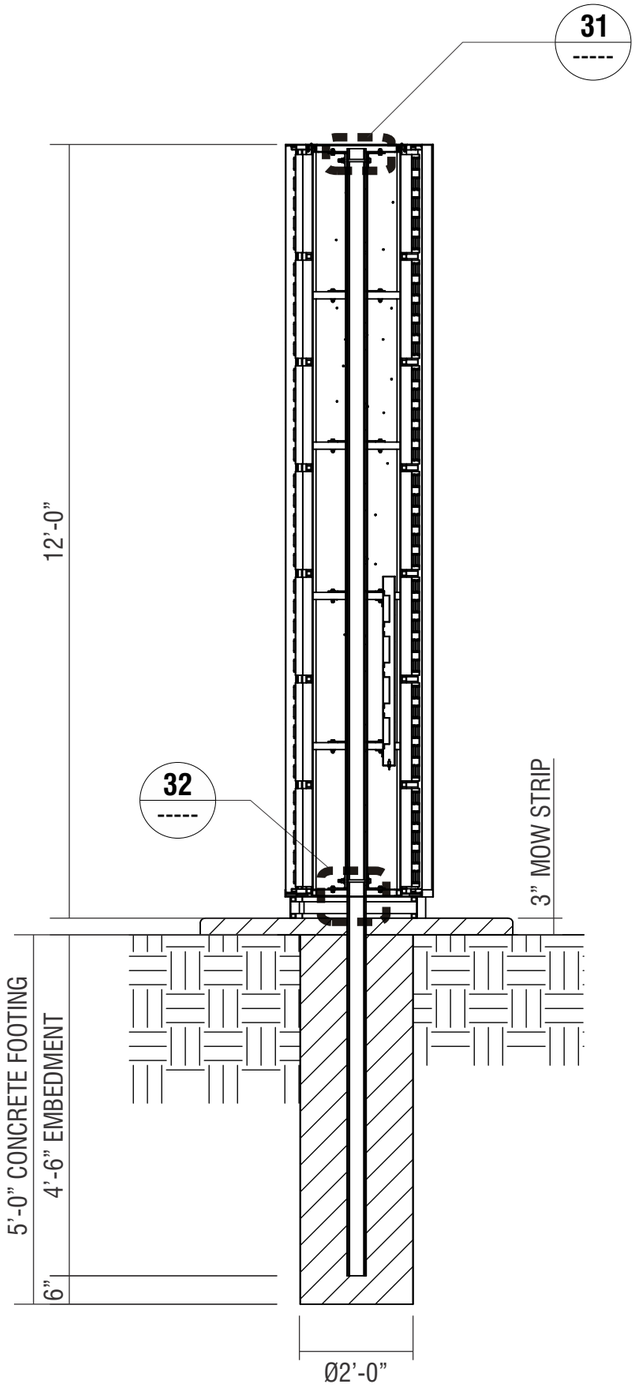
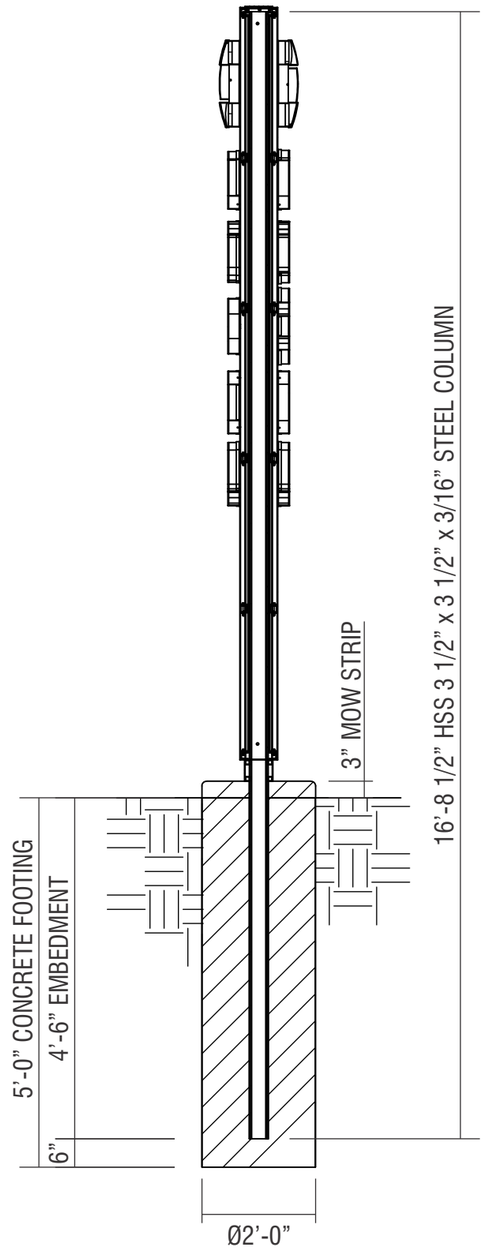
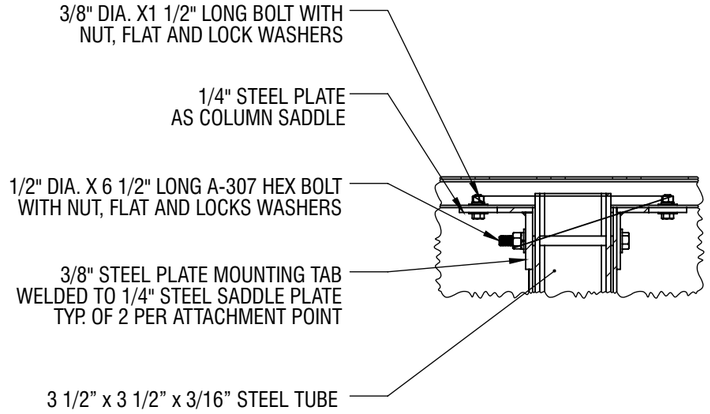


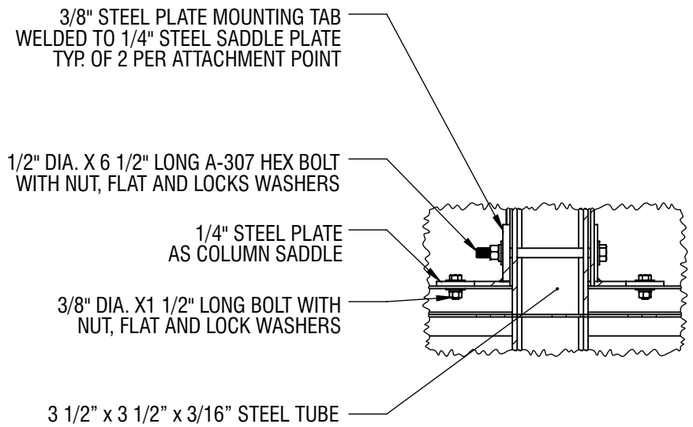
K-K LONGITUDINAL SECTION — Scale: 3/8" = 1'-0"



L-L VERTICAL SECTION — Scale: 3/8" = 1'-0"



31 TOP ATTACHMENT DETAIL — Scale: 1 ½" = 1'-0"



33 BOTTOM ATTACHMENT DETAIL — Scale: 1 ½" = 1'-0"

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GENERAL
1. ALL MATERIALS AND WORK SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 CALIFORNIA BUILDING CODE (CBC).
2. CONSTRUCTION METHODS AND PROJECT SAFETY: DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE METHODS, PROCEDURES, OR SEQUENCE OF CONSTRUCTION. TAKE NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE DURING CONSTRUCTION. THE EOR WILL NOT ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT, AND MAINTAIN ALL SAFETY DEVICES AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS, AND REGULATIONS.
3. VERIFY ALL DIMENSIONS, ELEVATIONS AND SITE CONDITIONS PRIOR TO THE START OF CONSTRUCTION AND NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES OR INCONSISTENCIES THAT ARE FOUND. NOTED DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS. DO NOT SCALE DRAWINGS.
4. ALL OMISSIONS AND/OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND FIELD INSPECTOR. THE ENGINEER SHALL PROVIDE A SOLUTION PRIOR TO PROCEEDING WITH ANY WORK AFFECTED BY THE CONFLICT OR OMISSION.
5. WHERE NO CONSTRUCTION DETAILS ARE SHOWN OR NOTED FOR ANY PART OF THE WORK, USE THOSE FOR OTHER SIMILAR WORK.
6. WHEN A DETAIL IS IDENTIFIED AS TYPICAL, APPLY IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE REFERENCE IS REPEATED IN EVERY INSTANCE.
7. CHANGES TO THE DRAWINGS: OBTAIN PRIOR WRITTEN APPROVAL.
8. WORK PERFORMED IN CONFLICT WITH THE DRAWINGS OR APPLICABLE BUILDING CODE REQUIREMENTS SHALL BE CORRECTED AT THE EXPENSE OF THE CONTRACTOR.

DESIGN CRITERIA
1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH ASCE 7-16: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.
2. WIND LOAD:
   BASIC WIND SPEED, V_{ULT} = 95 MPH MAXIMUM
   RISK CATEGORY: II EXPOSURE: C
3. SNOW LOAD:
   IMPORTANCE FACTOR, I_S = 1.0
   SURFACE ROUGHNESS: C EXPOSURE: C
   GROUND 0 PSF MAXIMUM
   ROOF --- PSF
4. ROOF LIVE LOAD: --- PSF
5. SEISMIC CRITERIA:
   SITE CLASS: D - DEF RISK CAT: II SDC: D
   S_S = 2.330 F_A = 1.200
   S_1 = 0.869 F_V = 1.700
   IMPORTANCE FACTOR, I_E = 1.0
   RESPONSE MODIFICATION FACTOR, R_p = 3.0
   OVERSTRENGTH FACTOR, Ω_0 = 1.75
   DEFLECTION AMPLIFICATION FACTOR, C_d = 3.0

STEEL
1. STEEL SHAPES SHALL CONFORM TO THE FOLLOWING (U.N.O.):

RND. HSS	ASTM A500, GR C	Fy=46 KSI MIN.
SQ./RECT. HSS	ASTM A500, GR C	Fy=50 KSI MIN.
THREADED ROD	ASTM A36	Fy=36 KSI MIN.
STEEL PLATE	ASTM A36	Fy=36 KSI MIN.
ANGLE ‡ CHANNEL	ASTM A36	Fy=36 KSI MIN.
STD. PIPE	ASTM A53, GR B	Fy=35 KSI MIN.
STRUCT. PIPE	ASTM A252, GR 3	Fy=45 KSI MIN.
WIDE FLANGE	ASTM A992	Fy=50 KSI MIN.

2. MACHINE BOLTS SPECIFIED AS "A307" SHALL CONFORM TO ASTM A307 w/ NUTS PER ASTM A563A ‡ WASHERS PER ASTM F844 (U.N.O.). THREADED PARTS, NUTS, AND WASHERS SHALL BE HDG OR ZP AS DEFINED HEREIN.
3. STRUCTURAL BOLTS SHALL CONFORM TO ASTM F3125 GRADES A325 OR A490 AS SPECIFIED ("A325" OR "A490") w/ NUTS PER ASTM A563DH ‡ WASHERS PER ASTM F436.
   A. WHERE DESIGNATED AS "-X", CARE MUST BE TAKEN TO ENSURE THREADS ARE EXCLUDED FROM THE SHEAR PLANE(S).
   B. WHERE DESIGNATED AS "-N" OR IF NO DESIGNATION IS NOTED, THREADS MAY BE INCLUDED IN THE SHEAR PLANE(S).
   C. WHERE SPECIFIED, "A325" MAY BE HDG OR ZP AS DEFINED HEREIN.
   D. GRADE "A490" SHALL NOT BE HDG OR ZP AS DEFINED HEREIN.
4. ANCHORS CAST IN CONCRETE SHALL CONFORM TO ASTM F1554 GR. 36 (U.N.O.) w/ NUTS TO ASTM A563 AND WASHERS TO ASTM F436. PARTS SHALL BE HOT-DIP GALVANIZED (HDG) OR ZINC (MECHANICAL) PLATED (ZP). PARTS EMBEDDED ENTIRELY IN CONCRETE MAY BE PLAIN STEEL.
5. WHERE SPECIFIED FOR STEEL THREADED PARTS, NUTS, AND WASHERS, HOT-DIP GALVANIZING (HDG) SHALL CONFORM TO ASTM F2329 AND ZINC (MECHANICAL) PLATING (ZP) TO CLASS 55 PER ASTM B695.
6. PLAIN STEEL FASTENERS ARE NOT TO BE USED UNLESS SPECIFIED.
7. ZINC ELECTRO-PLATED FASTENERS PER ASTM F1941 MAY BE SUBSTITUTED FOR INTERIOR APPLICATIONS, BUT ARE OTHERWISE NOT TO BE USED UNLESS SPECIFIED.
8. NUTS AND WASHERS SHALL HAVE THE SAME COATING AS THE CORRESPONDING THREADED PART.
9. WHERE SPECIFIED, IRON AND STEEL HARDWARE SHALL BE HOT-DIP GALVANIZED PER ASTM A153.
10. STAINLESS STEEL (SS) BOLTS, STUDS, AND THREADED ROD SHALL CONFORM TO ASTM F593 AND BE ALLOY 304 OR 316 w/ NUTS TO ASTM F594. NUTS AND WASHERS SHALL MATCH THE ALLOY OF THE THREADED PART.
11. WELDING:
    A. WELD STRUCTURAL STEEL IN COMPLIANCE WITH ANSI/AWS D1.1 AND AISC SPECIFICATION, CHAPTER J. WELDERS SHALL BE CERTIFIED AS REQUIRED BY THE LOCAL BUILDING AUTHORITY. WELDING SHALL BE DONE BY ELECTRIC ARC PROCESS USING LOW-HYDROGEN ELECTRODES WITH SPECIFIED TENSILE STRENGTH NOT LESS THAN 70 KSI UNLESS NOTED OTHERWISE.
    B. UNLESS A LARGER WELD SIZE IS INDICATED, PROVIDE MINIMUM SIZE WELD PER AISC SPECIFICATION, SECTION J2, TABLE J2.4.

ALUMINUM
1. FABRICATE AND ERECT ALUMINUM IN COMPLIANCE WITH THE 2020 ALUMINUM DESIGN MANUAL (ADM1), THE SPECIFICATIONS FOR ALUMINUM SHEET METAL WORK (ASM35), AND CHAPTER 20 OF THE BUILDING CODE.
2. ALUMINUM SHAPES SHALL CONFORM TO THE FOLLOWING:

PIPE ‡ TUBE	6061-T6	ASTM B429	Fy=35 KSI MIN.
STRUCT. PROFILES	6061-T6	ASTM B308	Fy=35 KSI MIN.
SHEET ‡ PLATE	6061-T6	ASTM B209	Fy=35 KSI MIN.
STAPLE TUBE	6063-T5	ASTM B221	Fy=16 KSI MIN.

3. ALL SHOP AND FIELD WELDS SHALL BE PERFORMED BY AN AISC QUALITY CERTIFIED FABRICATOR.
4. UNLESS A LARGER WELD SIZE IS INDICATED, PROVIDE MINIMUM SIZE WELD PER ADM 1.
5. FILLER SHALL BE 5556 ALLOY REGARDLESS OF MEMBER THICKNESS. NO OTHER FILLER ALLOY SHALL BE USED UNLESS NOTED OTHERWISE.

CONCRETE ‡ REINFORCEMENT
1. MINIMUM 28-DAY COMPRESSIVE STRENGTH (f'c) SHALL BE 2,500 PSI.
2. REINFORCEMENT TO BE ASTM A615 GR 60, Fy=60 KSI UNO.
3. CALCIUM CHLORIDE OR ADDED CHLORIDE IS NOT PERMITTED.
4. ALL REINFORCED CONCRETE SHALL BE CONSOLIDATED WITH MECHANICAL VIBRATORS.
5. MINIMUM CONCRETE COVER:
   CAST AGAINST ‡ EXPOSED TO EARTH 3"
   EXPOSED TO EARTH OR WEATHER 2"
6. CHAIRS AND SPACERS: AS REQUIRED TO MAINTAIN COVER.
7. SIGN MAY BE INSTALLED ON FOUNDATION AFTER A MINIMUM CURING TIME OF (14) DAYS PROVIDED CURING PROCESS IS PROPERLY MAINTAINED PER ACI 318 AND 308.
8. GROUT SHALL BE NON-SHRINK AND NON-METALLIC WITH A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI AT (1) DAY. MIX AND PLACE IN ACCORDANCE WITH MANUFACTURER INSTRUCTIONS.

FOUNDATIONS
1. DESIGN BEARING PRESSURES ARE PER CBC CLASS 5 PRESUMPTIVE VALUES (NO SPECIAL INSPECTION REQUIRED):
   LATERAL BEARING: 100 PSF/FT
   VERTICAL BEARING: 1,500 PSF

EXISTING CONDITIONS
1. ENGINEER WILL NOT BE PERFORMING ON-SITE INSPECTIONS OR VERIFICATIONS. IT IS THE RESPONSIBILITY OF THE INSTALLER AND OWNER(S) TO IDENTIFY EXISTING CONDITIONS AND CONTACT ENGINEER WITH ANY DISCREPANCIES OR CONCERNS.
2. EXISTING INFORMATION HAS BEEN FURNISHED BY THE ENTITY WHOM THIS DOCUMENT WAS PREPARED FOR. ENGINEER IN NO WAY CERTIFIES THIS INFORMATION AS "AS-BUILT".
3. FEATURES OF WORK ANNOTATED AS "VERIFY" (OR SIMILAR) MUST BE INSPECTED, VERIFIED AS SUCH, AND DOCUMENTED PRIOR TO FABRICATION AND INSTALLATION.
4. IF THERE IS ANY REASON TO BELIEVE THE EXISTING CONDITIONS DETAILED HEREIN ARE NOT ACCURATE, CONTRACTOR SHALL CEASE WORK AND NOTIFY ENGINEER IMMEDIATELY.
5. CONTRACTOR SHALL INSPECT AND CONFIRM THE QUALITY OF EXISTING STRUCTURE AS "IN GOOD REPAIR". STRUCTURE SHALL BE FREE OF CORROSION, DECAY, AND ANY OTHER MATERIAL, FABRICATION, ASSEMBLY, OR INSTALLATION DEFECT. IF THERE ARE ANY INDICATIONS THAT THIS IS NOT THE CASE, CONTRACTOR SHALL CEASE WORK IMMEDIATELY AND NOTIFY ENGINEER.

EVALUATION REPORT SCHEDULE
ANCHORS, FASTENERS, AND OTHER PRODUCTS SHALL CONFORM TO AND BE INSTALLED PER THEIR RESPECTIVE EVALUATION REPORT(S) AS FOLLOWS (NOT ALL APPLICABLE THIS PROJECT):

ANCHOR TYPE	REPORT #
HILTI KB-TZ2 (CS ‡ SS) ANCHORS IN CONCRETE	ICC-ESR-4266
HILTI KB-TZ2 (CS ‡ SS) ANCHORS IN MASONRY	ICC-ESR-4561
HILTI KH-EZ (CS ‡ SS) ANCHORS IN CONCRETE	ICC-ESR-3027
HILTI KH-EZ (CS ‡ SS) ANCHORS IN MASONRY	ICC-ESR-3056
HILTI HIT-HY 200 ADHESIVE IN CONCRETE	ICC-ESR-3187
HILTI HIT-HY 200 ADHESIVE IN MASONRY	ICC-ESR-3963
SIMPSON TITEN HD (CS) ANCHORS IN CONCRETE	ICC-ESR-2713
SIMPSON TITEN HD (CS ‡ SS) ANCHORS IN MASONRY	ICC-ESR-1056
SIMPSON TITEN HD (SS) ANCHORS IN CONCRETE	UES-ER-493
TAPCON ANCHORS IN MASONRY	ICC-ESR-1671
TAPCON ANCHORS IN CONCRETE	ICC-ESR-2202
TAPCON+ SCREW ANCHORS IN CONCRETE	ICC-ESR-3699
ITW BUILDEX TEKS SDS	ICC-ESR-1976

ABBREVIATIONS

ABV.	ABOVE
ADD'L.	ADDITIONAL
AFF	ABOVE FINISHED FLOOR
ALT.	ALTERNATE
ALUM.	ALUMINUM
A.O.R.	ARCHITECT OF RECORD
ARCH.	ARCHITECTURAL
BTM.	BOTTOM
BLK'G.	BLOCKING
CIRC.	CIRCLE/CIRCULAR
CONC.	CONCRETE
CONN.	CONNECTION
CONT.	CONTINUOUS
CTR.	CONTRACTOR
DIA.	DIAMETER
DET.	DETAIL
(E)	EXISTING
EXIST.	EXISTING
EA.	EACH
E.W.	EACH WAY
ELEV.	ELEVATION
EMBED.	EMBEDMENT
E.O.R.	ENGINEER OF RECORD
FAB.	FABRICATOR/FABRICATION
FDN.	FOUNDATION
FRM'G.	FRAMING
FTG.	FOOTING
F.V.	FIELD VERIFY
G.C.	GENERAL CONTRACTOR
HDG	HOT DIP GALVANIZED
HOR.	HORIZONTAL
O.C.	ON CENTER
LOC.	LOCATION
MAX.	MAXIMUM
MIN.	MINIMUM
(N)	NEW
N.T.E.	NOT TO EXCEED
o/	OVER
O.D.	OUTSIDE DIAMETER
OPT.	OPTIONAL
PENE.	PENETRATION
REINF.	REINFORCEMENT
RND	ROUND
SIM.	SIMILAR
SS	STAINLESS STEEL
STD	STANDARD
SUPP.	SUPPLEMENTAL
SQ.	SQUARE
T/O	TOP OF
TYP.	TYPICAL
THK.	THICK(NESS)
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
w/	WITH
w/o	WITHOUT
ZP	ZINC (MECHANICAL) PLATED

THIS AREA INTENTIONALLY LEFT BLANK

MANUFACTURED SIGN CABINETS
UNLESS NOTED OTHERWISE, MANUFACTURED SIGN CABINETS SHALL BE DESIGNED BY THE MANUFACTURER/FABRICATOR OR OTHER COMPETENT PARTY AND FABRICATED IN ACCORDANCE WITH ALL APPLICABLE CODES, UL LISTINGS, LOCAL ORDINANCES, AND INDUSTRY STANDARDS. THIS INCLUDES FACES AND CLADDING, INTERNAL STRUCTURE, ELECTRICAL, AND ALL OTHER ACCESSORY COMPONENTS.

THE MANUFACTURER/FABRICATOR IS RESPONSIBLE FOR ENSURING ALL CABINETS ARE ASSEMBLED WITH ADEQUATE INTERNAL FRAMING AND STIFFNESS. CABINET FRAMING SHALL BE CAPABLE OF DELIVERING ALL IMPOSED DESIGN LOADS (WIND, SEISMIC, DEAD, SNOW, ETC.) DIRECTLY TO THE STRUCTURAL CONNECTIONS OR ELEMENTS DETAILED HEREIN. CABINET FRAMING SHALL LIMIT EXCESSIVE VIBRATION, DRIFT, OR DEFLECTION TO REASONABLE LEVELS.

FAILURE TO PROVIDE AN ADEQUATE LOAD PATH OR SUFFICIENT CABINET STIFFNESS MAY RESULT IN EXCESSIVE VIBRATION, DRIFT, OR DEFLECTION WHICH MAY YIELD SECOND-ORDER EFFECTS THAT CAN NEGATIVELY AFFECT THE PERFORMANCE OF THE STRUCTURAL CONNECTIONS OR ELEMENTS DETAILED HEREIN.

REVERENCE ENGINEERING MAKES NO CLAIMS AS TO THE SUITABILITY OF MANUFACTURED SIGN CABINETS IDENTIFIED AS "BY MFR." OR "BY FAB." WHICH HAVE NOT BEEN ENGINEERED, CERTIFIED, OR REVIEWED BY REVERENCE ENGINEERING UNLESS SPECIFICALLY CONTRACTED OTHERWISE AND DETAILED OR NOTED HEREIN.

DESIGN BY OTHERS NOTE
REVERENCE ENGINEERING IN NO WAY CERTIFIES OR MAKES CLAIMS TO THE SUITABILITY OF CONDITIONS OR ELEMENTS (EXISTING OR NEW) THAT ARE DESIGNED BY OTHERS. SUCH CONDITIONS AND ELEMENTS ARE IDENTIFIED AS "BY OTHERS" OR "DESIGN(ED) BY OTHERS" AND ARE NOT ENGINEERED BY REVERENCE ENGINEERING.

THE SCOPE OF ENGINEERING HEREIN ASSUMES THESE ELEMENTS HAVE BEEN, OR WILL BE, DESIGNED OR CHECKED FOR SUITABILITY BY A DESIGN PROFESSIONAL.

CONNECTION TO EXISTING STRUCTURE
REVERENCE ENGINEERING IN NO WAY CERTIFIES THE EXISTING STRUCTURE AS ADEQUATE AND ABLE TO SUPPORT THE LOADS FROM THE ASSEMBLY DETAILED HEREIN.

REVERENCE ENGINEERING HAS PROVIDED THESE DRAWINGS WITH THE UNDERSTANDING THAT THE EXISTING STRUCTURE WAS EITHER ORIGINALLY DESIGNED TO ACCEPT THE ASSEMBLY DETAILED HEREIN OR HAS BEEN (OR WILL BE) ASSESSED FOR ADEQUACY PRIOR TO FABRICATION AND INSTALLATION. IT IS THE UNDERSTANDING OF REVERENCE ENGINEERING THAT SUCH DETERMINATION OR EVALUATION HAS BEEN OR WILL BE MADE KNOWN TO THE OWNER/CONTRACTOR/FABRICATOR/SUB-CONTRACTOR.

ELECTRICAL NOTE
ELECTRIC COMPONENTS AND WIRING ARE NOT DESIGNED BY REVERENCE ENGINEERING. FABRICATOR AND INSTALLER SHALL COMPLY WITH THE CURRENT VERSION OF THE ADOPTED NATIONAL ELECTRIC CODE (NEC) AND ARTICLE 600: "ELECTRIC SIGNS AND OUTLINE LIGHTING".

Limits of liability extend only to the work detailed, for the quantity of assemblies indicated (1 unless noted otherwise), at the location specified, and by the client listed; use of these plans and/or corresponding structural calculations in violation of either voids all liability.

REVERENCE ENGINEERING
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PREPARED FOR:
SIGNTECH ELECTRICAL ADVERTISING, INC.

PROJECT #:
2405059

CHASE
48600R010443
PYLON SIGN

494 TRES PINOS RD.
HOLLISTER, CA 95023

No:	Issue/Revision:	Date:
----	Initial Submittal	05-22-2024
1		
2		
3		
4		

REGISTERED PROFESSIONAL ENGINEER
BRETT SAUERS
No. C81435
CIVIL
STATE OF CALIFORNIA
22-May-24

SHEET TITLE:
STRUCTURAL

SHEET:
S.1

ORIGINAL SHEET SIZE: 11x17

Signtech
4444 Federal Blvd. San Diego CA 92102
Phone: (619) 527-6100 / Fax: (619) 527-6111
signtech.com
MEMBER WSA ISA CESA

JP Morgan Chase Bank #48600R010443

Hollister
494 Tres Pinos Rd
Hollister, CA 95023

Initial Date: 08/11/23
Salesperson: Arthur Navarro
Coordinator: Fabian Marquez
Designer: ASena
Scale: As noted

CUSTOMER APPROVAL

Customer Signature ______ Date ______

COPY, COLORS & SIZES

Signtech does NOT provide primary electrical to sign location - ***RESPONSIBILITY OF OTHERS!***

Customer Signature ______ Date ______

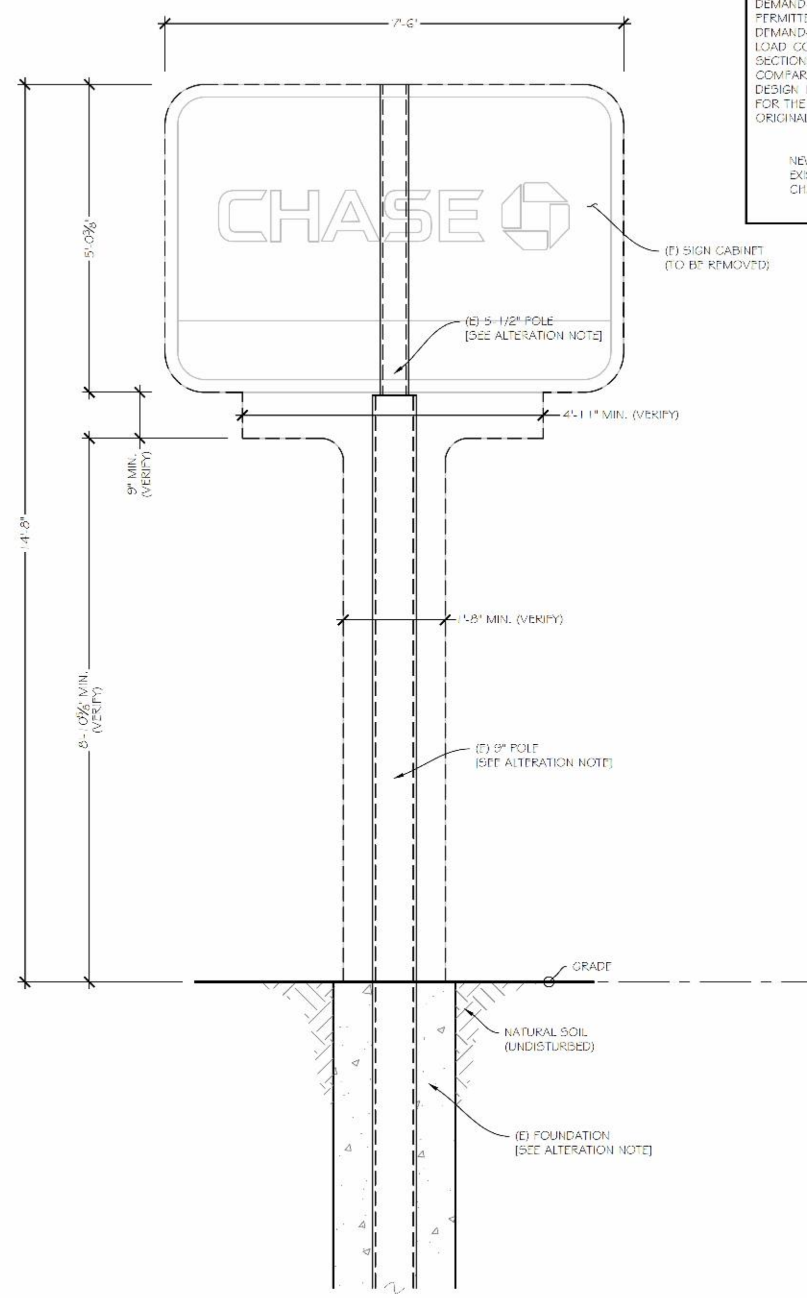
Drawing Number: 23-01425
Project ID: CHASE_48600R010443_1
Revision: PERMIT-PYL-R2: AJK - 09/02/25

EBC SECTION 503 - ALTERATIONS

EXISTING STRUCTURAL ELEMENTS CARRYING LATERAL LOAD. EXCEPT AS PERMITTED BY SECTION 503.13, WHERE THE ALTERATION INCREASES DESIGN LATERAL LOADS IN ACCORDANCE WITH SECTION 1609 OR 1613, OR WHERE THE ALTERATION RESULTS IN A STRUCTURAL IRREGULARITY AS DEFINED IN ASCE 7, OR WHERE THE ALTERATION DECREASES THE CAPACITY OF ANY EXISTING LATERAL LOAD-CARRYING STRUCTURAL ELEMENT, THE STRUCTURE OF THE ALTERED BUILDING OR STRUCTURE SHALL BE SHOWN TO MEET THE REQUIREMENTS OF SECTIONS 1609 AND 1613.

EXCEPTION: ANY EXISTING LATERAL LOAD-CARRYING STRUCTURAL ELEMENT WHOSE DEMAND-CAPACITY RATIO WITH THE ALTERATION CONSIDERED IS NO MORE THAN 10 PERCENT GREATER THAN ITS DEMAND-CAPACITY RATIO WITH THE ALTERATION IGNORED SHALL BE PERMITTED TO REMAIN UNALTERED. FOR PURPOSES OF CALCULATING DEMAND-CAPACITY RATIOS, THE DEMAND SHALL CONSIDER APPLICABLE LOAD COMBINATIONS WITH DESIGN LATERAL LOADS OF FORCES PER SECTIONS 1609 AND 1613. FOR PURPOSES OF THIS EXCEPTION, COMPARISONS OF DEMAND-CAPACITY RATIOS AND CALCULATION OF DESIGN LATERAL LOADS, FORCES, AND CAPACITIES SHALL ACCOUNT FOR THE CUMULATIVE EFFECTS OF ADDITIONS AND ALTERATIONS SINCE ORIGINAL CONSTRUCTION.

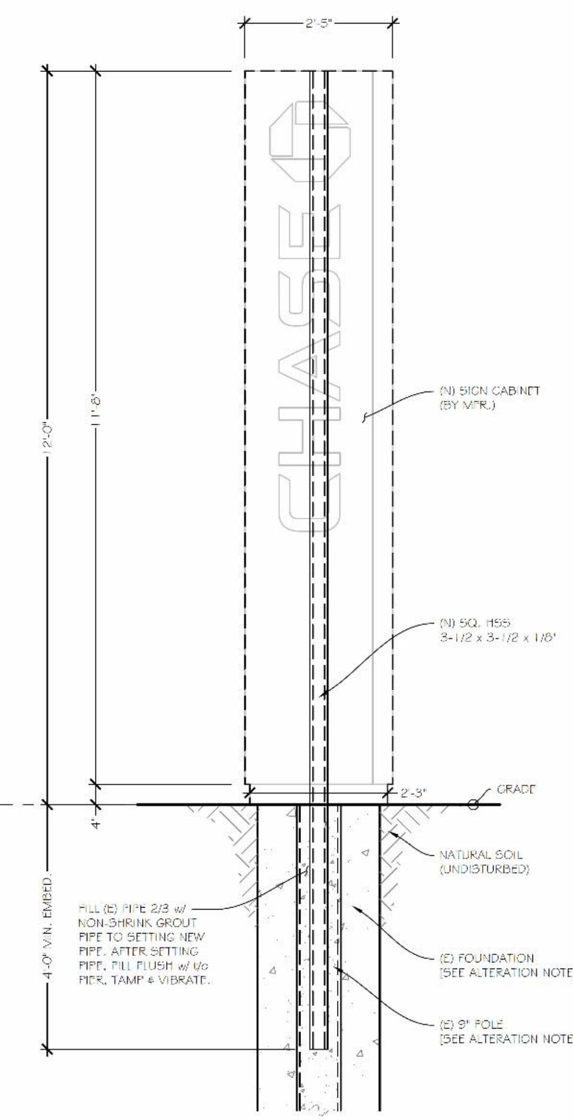
	SHEAR		FLEXURE		TORSION	
NEW	0.63	KIP	4.15	K-FT	0.30	K-FT
EXISTING	1.31	KIP	13.51	K-FT	1.59	K-FT
CHANGE	-52.1	%	-69.3	%	-81.0	%



1 EXISTING SIGN 001 ELEVATION

CUT LINE

CUT LINE NOTE:
FIELD VERIFY (E) PIPE PRIOR TO CUTTING DOWN. SHOULD THICKNESS DIFFER FROM AS NOTED, CONTACT E.O.R. FOR REDESIGN



2 PROPOSED SIGN 001 ELEVATION

RE
REVERENCE ENGINEERING
www.reverenceengineering.com
(619) 354-1152
501 W BROADWAY, STE 425
SAN DIEGO, CA 92101

PREPARED FOR:
SIGNTECH ELECTRICAL ADVERTISING, INC.

PROJECT #: 2405059

CHASE
48600R010443
PYLON SIGN
494 TRES PINOS RD.
HOLLISTER, CA 95023

No:	Issue/Revision:	Date:
----	Initial Submittal	05-22-2024
1		
2		
3		
4		

REGISTERED PROFESSIONAL ENGINEER
BRETT SAUERS
No. C81435
CIVIL
STATE OF CALIFORNIA
22 May 24

SHEET TITLE: STRUCTURAL

SHEET: S.2

ORIGINAL SHEET SIZE: 11x17

Signtech
4444 Federal Blvd. San Diego CA 92102
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WSA MEMBER ISA MEMBER OF CESA

JP Morgan Chase Bank #48600R010443

Hollister
494 Tres Pinos Rd
Hollister, CA 95023

Initial Date: 08/11/23
Salesperson: Arthur Navarro
Coordinator: Fabian Marquez
Designer: ASena
Scale: As noted

CUSTOMER APPROVAL

Customer Signature ____ Date ____

COPY, COLORS & SIZES

Signtech does NOT provide primary electrical to sign location - ***RESPONSIBILITY OF OTHERS!***

Customer Signature ____ Date ____

Drawing Number: 23-01425
Project ID: CHASE_48600R010443_1
Revision: PERMIT-PYL-R2: AJK - 09/02/25

REVERENCE ENGINEERING

STRUCTURAL CALCULATIONS

for

Chase 48600R010443 Pylon Sign

at

494 Tres Pinos Rd.

Hollister, CA 95023

Prepared for:

Signtech Electrical Advertising, Inc.

Package Type:	Project #:
Initial Submittal	**2405059**

DESIGN SPECIFICATIONS

1. California Building Code (CBC) 2022
2. ASCE 7-16: Minimum Design Loads for Buildings and Other Structures
3. ACI 318-19: Building Code Requirements for Structural Concrete
4. ANSI/AISC 360-16: Specification for Structural Steel Buildings
5. Aluminum Design Manual (ADM-1) 2020

DESIGN CRITERA

::Wind:: V_{ult} = 95 mph

Exposure: C

::Soils::

Per Building Code Presumptive Class 5

Allowable Lateral Bearing: 100 psf/ft

Allowable Vertical Bearing: 1500 psf

::Seismic Design Parameters::

Site Class: D - Default SDC: E

S_s = 2.330 g

S_1 = 0.869 g

T_L = 12 sec

Date Signed: 22/05/2024

Reverence Engineering

501 W Broadway, STE 425

San Diego, CA 92101

(o) 619-354-1152

projects@reverenceengineering.com

EXISTING SIGN 001

Signtech™

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MEMBER

MEMBER OF CESA

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CUSTOMER APPROVAL

Customer Signature Date

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Customer Signature Date

Drawing Number: 23-01425

Project ID: CHASE_48600R010443_1

Revision: PERMIT-PYL-R2: AJK - 09/02/25

Wind Pressure Analysis

Design Wind Loads on Solid Signs

Notes & Assumptions:

1. Wind Loads on other structures and building appurtenances per Chapter 29, Section 29.5 (Solid Signs).

V_{ult} =	95	mph	Basic Wind Speed	z_g =	900	ft	Terrain Exposure Constant [Table 26.9-1]
Exp:	C	-	Exposure Category	α =	9.5	-	Terrain Exposure Constant [Table 26.9-1]
ε =	100%	-	Solid to Gross Area Ratio (porosity)	K_d =	0.85		Directionality Factor [Table 26.6-1]
Add'l =	0%	-	Add'l. capacity applied to p_z	K_{zt} =	1		Topographic Factor [26.8.2]
b =	0	ft	2-pole spacing ('0' for one pole or other)	G =	0.85	-	Gust Effect Factor [26.9.1]

Cab. #	h (ft)	s (ft)	B (ft)	Encl.[1]	t^2 (ft)	s/h	B/s	R_{min}	R_{max}	~~~~~~ Cases A & B C_f Interpolation ~~~~~~						C_f	0.2B	Case B Distrib.
1	14.667	5.0313	7.5	Yes		0.34	1.49	0.00	0.00	1.75	1.70	1.80	1.80	1.73	1.80	1.78	1.50	**1.00**
2																		
3																		
4																		
5																		
6																		
7																		
8																		

[1] For double-faced signs with all sides enclosed select "Yes" for C_f reduction per code. ("No" or blank is conservative if unknown)

[2] Cabinet Thickness (Use 0 or leave blank for unknown)

Figure 29.4-1 C_f Cases A & B

s/h	Aspect Ratio B/s													
	0	0.05	0.1	0.2	0.5	1	2	4	5	10	20	30	45	100
1.00	1.80	1.80	1.70	1.65	1.55	1.45	1.40	1.35	1.35	1.30	1.30	1.30	1.30	1.30
0.90	1.85	1.85	1.75	1.70	1.60	1.55	1.50	1.45	1.45	1.40	1.40	1.40	1.40	1.40
0.70	1.90	1.90	1.85	1.75	1.70	1.65	1.60	1.60	1.55	1.55	1.55	1.55	1.55	1.55
0.50	1.95	1.95	1.85	1.80	1.75	1.75	1.70	1.70	1.70	1.70	1.70	1.70	1.75	1.75
0.30	1.95	1.95	1.90	1.85	1.80	1.80	1.80	1.80	1.80	1.80	1.85	1.85	1.85	1.85
0.20	1.95	1.95	1.90	1.85	1.80	1.80	1.80	1.80	1.80	1.85	1.90	1.90	1.95	1.95
0.16	1.95	1.95	1.90	1.85	1.85	1.80	1.80	1.85	1.85	1.85	1.90	1.90	1.95	1.95
0.00	1.95	1.95	1.90	1.85	1.85	1.80	1.80	1.85	1.85	1.85	1.90	1.90	1.95	1.95

h = Average Height (ft) at top of section/element
s = Vertical sign dimension (ft) `
B = Horizontal sign dimension (ft)
z = Centroid of Section Height
K_z = Velocity Pressure Exposure Coefficient
q_h = Velocity Pressure (psf)
p = Design Wind Pressure (psf)

s/h = Cabinet to Average Height Ratio
B/s = Cabinet Width to Cabinet Height Ratio

$q_z = 0.00256 * K_d * K_z * K_{zt} * V^2$

$p = q_h * G * C_f$

Wind Load Analysis

SIGN BUILD

FORCE is PER POLE and reflects distribution per Case B to applicable components

LOW - > HIGH

#	Δh (ft)	Width (ft)	Type	Cant.	Cab. #	h (ft)	z (ft)	C_f	K_z	q_z (psf)	p_z (psf)	A_F (sqft)	F_u (kip)	M_u (k-ft)	T_u (k-ft)	M_{u1}
1	8.8854	1.6667	Sq. Normal			**8.9**	4.4427	1.3722	0.8489	16.671	19.444	14.8	0.29	1.28	0.1	
2	0.75	4.9167	Sq. Normal			**9.6**	9.2604	1.316	0.8489	16.671	18.648	3.7	0.07	0.64	0.1	
3	5.0313	7.5	Cabinet		1	**14.7**	12.151	1.784	0.8489	16.671	25.279	37.7	0.95	11.59	1.4	
4																
5																
6																
7																
8																
9																
10																
11																
12																
13																
14																
15																
16																
17																
18																
19																
20																
21																
22																
23																
24																
25																

H = 14.667 ft	← Overall height check[1]	"Grade Reactions" (Intermediate Calculation) →	1.3 kip	13.5 kip-ft	1.6 kip-ft	0.0 kip-ft

[1] For evaluation, may not be equivalent to the actual structure height above grade

Splice or Section Loads / Reactions

Notes & Assumptions:

1. Moments this section include an additional 10% per 29.4-1 if s/h=1.
2. Moments and Shears include either 50% split or Distribution Factor in the case of 2 poles (torsion is zero).
3. Splice Reactions may be a physical splice in the system or any other point of interest

Splice	h (ft)	Stage	w (ft)	h+ (ft)	p (psf)	A_F (sqft)	F_u (kip)	z (ft)
1	9.5833	2	4.9167	9.6354	18.648	0.26	0.00	0.03
2								
3								
4								
5								
6								

	V (kip)	M (k-ft)	T (k-ft)
1	0.959	2.449	1.436
2			
3			
4			
5			
6			
GRADE REACTIONS* →	1.31	13.51	1.59

*Grade Reactions are not necessarily at physical grade, but represent the reactions at the lowest elevation (Elev. = 0'-0") calculated from the above "Sign Build" table.

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WSA WORLD SIGN ASSOCIATES
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JP Morgan Chase Bank #48600R010443

Hollister
494 Tres Pinos Rd
Hollister, CA 95023

Initial Date: 08/11/23
Salesperson: Arthur Navarro
Coordinator: Fabian Marquez
Designer: ASena
Scale: As noted

CUSTOMER APPROVAL

Customer Signature ____ Date ____

COPY, COLORS & SIZES

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Customer Signature ____ Date ____

Drawing Number: 23-01425
Project ID: CHASE_48600R010443_1
Revision: PERMIT-PYL-R2: AJK - 09/02/25

Seismic Load Analysis

SEISMIC ANALYSIS FOR NON-BUILDING STRUCTURES NOT SIMILAR TO BUILDINGS — REF: ASCE CHAPTER 15

Site Class:	D - Default			
S_s =	2.33	of g	Short Period Spectral Acceleration (USGS)	
S_1 =	0.869	of g	1-sec Period Spectral Acceleration (USGS)	
T_L =	12	sec	Long-Period Transition Period (USGS)	
I_e =	1.0	-	ASCE 7-16 Table 1.5-2 per 15.4.1.1	
h_n =	14.667	ft	Structural height	
w =	15	psf	Seismic Weight for analysis (conservative)	
R =	3	-	Response Modification Coefficient for: Signs and Billboards (Chapter 15)	
F_a =	1.20	-	Short-Period Site Coefficient	
F_v =	1.70	-	Long-Period Site Coefficient	
S_{MS} =	2.80	-	Spectral Response Acceleration Parameter	[Eqn 11.4-1]
S_{M1} =	1.48	-	Spectral Response Acceleration Parameter	[Eqn 11.4-2]
S_{DS} =	1.86	-	Design Spectral Acceleration Parameter	[Eqn 11.4-3]
S_{D1} =	0.98	-	Design Spectral Acceleration Parameter	[Eqn 11.4-4]
x =	0.75	-	Period Parameter	[Table 12.8-2]
C_t =	0.02	-	Period Parameter	[Table 12.8-2]
T_a =	0.15	sec	Approximate Fundamental Period	[Eqn 12.8-7]
T_s =	0.5284	sec		

SEISMIC RESPONSE COEFFICIENTS PER ASCE 7-16 12.8.1.1

C_s =	0.6213 -	Seismic Response Coefficient	[Eqn 12.8-2]
C_s =	2.1902 -	Upper limit for $T \le T_L$	[Eqn 12.8-3]
C_s =	175.34 -	Upper limit for $T > T_L$	[Eqn 12.8-4]
$C_{s,use}$ =	0.6213 -		

Table 11.4-1: Short-Period Site Coefficient, F_a (excerpt)

Site Class	Short Period Spectral Acceleration, S_s							
	0.25	0.5	0.75	1.0	1.25	1.5	1.50	1.50
D	1.6	1.4	1.2	1.1	1.0	1.0	1.00	1.00

Table 11.4-2: Long-Period Site Coefficient, F_v (excerpt)

Site Class	1-Sec Period Spectral Acceleration, S_1							
	0.1	0.2	0.3	0.4	0.5	0.6	0.60	0.60
D	2.4	2.2	2	1.9	1.8	1.7	1.70	1.70

Section 11.4.8 Exception:

2. Structures on Site Class D sites with S1 greater than or equal to 0.2, provided the value of the seismic response coefficient Cs is determined by Eq. (12.8-2) for values of T ≤ 1.5Ts and taken as equal to 1.5 times the value computed in accordance with either Eq. (12.8-3) for TL ≥ T > 1.5Ts or Eq. (12.8-4) for T > TL.

Values for Section 11.4.8 Exception (as required):

for	$T \le 1.5\,T_s$	C_s = 0.6213 -	<--Use
for	$1.5\,T_s < T \le T_L$	C_s = 3.2853 -	
for	$T > T_L$	C_s = 263.01 -	
		$C_{s,use}$ = 0.6213 -	

$C_{s,use}$ = 0.6213 - Seismic Response Coefficient

#	Δh (ft)	Width (ft)	Weight Overrides W (lb)	w (plf)	w (psf)	h (ft)	z (ft)	W (lb)	F_u (kip)	M_u (k-ft)
1	8.9	1.7				8.9	4.4427	222.14	0.138	0.6132
2	0.8	4.9				9.6354	9.2604	55.313	0.0344	0.3183
3	5.0	7.5				14.667	12.151	566.02	0.3517	4.2733
4						14.667	14.667	0	0	0
5						14.667	14.667	0	0	0
6						14.667	14.667	0	0	0
7						14.667	14.667	0	0	0
8						14.667	14.667	0	0	0
9						14.667	14.667	0	0	0
10						14.667	14.667	0	0	0
11						14.667	14.667	0	0	0
12						14.667	14.667	0	0	0
13						14.667	14.667	0	0	0
14						14.667	14.667	0	0	0
15						14.667	14.667	0	0	0
16						14.667	14.667	0	0	0
17						14.667	14.667	0	0	0
18						14.667	14.667	0	0	0
19						14.667	14.667	0	0	0
20						14.667	14.667	0	0	0
H =	14.667 ft		Overall height check							

Seismic Load (per pole):
Base Bending Moment: 5.2 k-ft
Base Shear: 0.5241 kip
WIND GOVERNS

Overstrength (per pole): Ω = 1.75 -
Base Bending Moment: 9.1 k-ft
Base Shear: 0.9171 kip
WIND GOVERNS

Post-Installed Anchors (0.75) per 17.2.3.4.4:
(only when applicable)
Base Bending Moment: 12.1 k-ft
Base Shear: 1.2 kip
WIND GOVERNS

Project #: 2405059 | Chase 48600R010443 Pylon Sign | 494 Tres Pinos Rd., Hollister, CA 95023 | REVERENCE ENGINEERING

PROPOSED SIGN 001

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Chase 48600R010443 Pylon Sign

494 Tres Pinos Rd., Hollister, CA 95023

REVERENCE
ENGINEERING

Wind Pressure Analysis

Design Wind Loads on Solid Signs

Notes & Assumptions:

1. Wind Loads on other structures and building appurtenances per Chapter 29, Section 29.5 (Solid Signs).

V_{ult} =	95	mph	Basic Wind Speed	z_g =	900	ft	Terrain Exposure Constant [Table 26.9-1]
Exp:	C	-	Exposure Category	α =	9.5	-	Terrain Exposure Constant [Table 26.9-1]
ε =	100%	-	Solid to Gross Area Ratio (porosity)	K_d =	0.85		Directionality Factor [Table 26.6-1]
Add'l =	0%	-	Add'l. capacity applied to p_z	K_{zt} =	1		Topographic Factor [26.8.2]
b =	0	ft	2-pole spacing ('0' for one pole or other)	G =	0.85	-	Gust Effect Factor [26.9.1]

Cab. #	h (ft)	s (ft)	B (ft)	Encl.[1]	t[2] (ft)	s/h	B/s	R_{min}	R_{max}	~~~~~~ Cases A & B C_f Interpolation ~~~~~~						C_f	0.2B	Case B Distrib.
1	12	12	2.4167	Yes	1.3125	1.00	0.20	0.54	0.11	1.65	1.55	1.70	1.60	1.65	1.70	1.53	0.42	**1.00**
2																		
3																		
4																		
5																		
6																		
7																		
8																		

[1] For double-faced signs with all sides enclosed select "Yes" for C_f reduction per code. ("No" or blank is conservative if unknown)

[2] Cabinet Thickness (Use 0 or leave blank for unknown)

Figure 29.4-1 C_f Cases A & B

s/h	Aspect Ratio B/s													
	0	0.05	0.1	0.2	0.5	1	2	4	5	10	20	30	45	100
1.00	1.80	1.80	1.70	1.65	1.55	1.45	1.40	1.35	1.35	1.30	1.30	1.30	1.30	1.30
0.90	1.85	1.85	1.75	1.70	1.60	1.55	1.50	1.45	1.45	1.40	1.40	1.40	1.40	1.40
0.70	1.90	1.90	1.85	1.75	1.70	1.65	1.60	1.60	1.55	1.55	1.55	1.55	1.55	1.55
0.50	1.95	1.95	1.85	1.80	1.75	1.75	1.70	1.70	1.70	1.70	1.70	1.70	1.75	1.75
0.30	1.95	1.95	1.90	1.85	1.80	1.80	1.80	1.80	1.80	1.80	1.85	1.85	1.85	1.85
0.20	1.95	1.95	1.90	1.85	1.80	1.80	1.80	1.80	1.80	1.85	1.90	1.90	1.95	1.95
0.16	1.95	1.95	1.90	1.85	1.85	1.80	1.80	1.85	1.85	1.85	1.90	1.90	1.95	1.95
0.00	1.95	1.95	1.90	1.85	1.85	1.80	1.80	1.85	1.85	1.85	1.90	1.90	1.95	1.95

h = Average Height (ft) at top of section/element
s = Vertical sign dimension (ft)
B = Horizontal sign dimension (ft)
z = Centroid of Section Height
K_z = Velocity Pressure Exposure Coefficient
q_h = Velocity Pressure (psf) $q_z = 0.00256*K_d*K_z*K_{zt}*V^2$
p = Design Wind Pressure (psf) $p = q_h*G*C_f$

s/h = Cabinet to Average Height Ratio
B/s = Cabinet Width to Cabinet Height Ratio

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Project #:

2405059

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REVERENCE
ENGINEERING

Wind Load Analysis

SIGN BUILD

FORCE is PER POLE and reflects distribution per Case B to applicable components

LOW - - - - > HIGH

#	Δh (ft)	Width (ft)	Type	Cant.	Cab. #	h (ft)	z (ft)	C_f	K_z	q_z (psf)	p_z (psf)	A_F (sqft)	F_u (kip)	M_u (k-ft)	T_u (k-ft)	M_{u1}
1	0.3333	2.25	Cabinet		1	**0.3**	0.1667	1.5304	0.8489	16.671	21.686	0.8	0.02	0.00	0.0	
2	11.667	2.4167	Cabinet		1	**12.0**	6.1667	1.5304	0.8489	16.671	21.686	28.2	0.61	3.77	0.3	
3																
4																
5																
6																
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22																
23																
24																
25																
H = 12 ft	← Overall height check[1]									"Grade Reactions" (Intermediate Calculation) →			0.6 kip	3.8 kip-ft	0.3 kip-ft	0.0 kip-ft

[1] For evaluation, may not be equivalent to the actual structure height above grade

Splice or Section Loads / Reactions

Notes & Assumptions:

1. Moments this section include an additional 10% per 29.4-1 if s/h=1.
2. Moments and Shears include either 50% split or Distribution Factor in the case of 2 poles (torsion is zero).
3. Splice Reactions may be a physical splice in the system or any other point of interest

Splice	h (ft)	Stage	w (ft)	h+ (ft)	p (psf)	A_F (sqft)	F_u (kip)	z (ft)
1								
2								
3								
4								
5								
6								

V (kip)	M (k-ft)	T (k-ft)
0.63	4.15	0.30

GRADE REACTIONS* → 0.63 | 4.15 | 0.30

*Grade Reactions are not necessarily at physical grade, but represent the reactions at the lowest elevation (Elev. = 0'-0") calculated from the above "Sign Build" table.

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REVERENCE ENGINEERING

Seismic Load Analysis

SEISMIC ANALYSIS FOR NON-BUILDING STRUCTURES NOT SIMILAR TO BUILDINGS — REF: ASCE CHAPTER 15

Site Class:	D - Default				
S_s =	2.33	of g	Short Period Spectral Acceleration (USGS)		
S_1 =	0.869	of g	1-sec Period Spectral Acceleration (USGS)		
T_L =	12	sec	Long-Period Transition Period (USGS)		
I_e =	1.0	-	ASCE 7-16 Table 1.5-2 per 15.4.1.1		
h_n =	12	ft	Structural height		
w =	15	psf	Seismic Weight for analysis (conservative)		
R =	3	-	Response Modification Coefficient for:		
			Signs and Billboards (Chapter 15)		
F_a =	1.20	-	Short-Period Site Coefficient		
F_v =	1.70	-	Long-Period Site Coefficient		
S_{MS} =	2.80	-	Spectral Response Acceleration Parameter	[Eqn 11.4-1]	
S_{M1} =	1.48	-	Spectral Response Acceleration Parameter	[Eqn 11.4-2]	
S_{DS} =	1.86	-	Design Spectral Acceleration Parameter	[Eqn 11.4-3]	
S_{D1} =	0.98	-	Design Spectral Acceleration Parameter	[Eqn 11.4-4]	
x =	0.75	-	Period Parameter	[Table 12.8-2]	
C_t =	0.02	-	Period Parameter	[Table 12.8-2]	
T_a =	0.13	sec	Approximate Fundamental Period	[Eqn 12.8-7]	
T_s =	0.5284	sec			

SEISMIC RESPONSE COEFFICIENTS PER ASCE 7-16 12.8.1.1

C_s =	0.6213 -	Seismic Response Coefficient	[Eqn 12.8-2]
C_s =	2.5459 -	Upper limit for $T \le T_L$	[Eqn 12.8-3]
C_s =	236.92 -	Upper limit for $T > T_L$	[Eqn 12.8-4]
$C_{s,use}$ =	0.6213 -		

Table 11.4-1: Short-Period Site Coefficient, F_a (excerpt)

Site Class	Short Period Spectral Acceleration, S_s							
	0.25	0.5	0.75	1.0	1.25	1.5	1.50	1.50
D	1.6	1.4	1.2	1.1	1.0	1.0	1.00	1.00

Table 11.4-2: Long-Period Site Coefficient, F_v (excerpt)

Site Class	1-Sec Period Spectral Acceleration, S_1							
	0.1	0.2	0.3	0.4	0.5	0.6	0.60	0.60
D	2.4	2.2	2	1.9	1.8	1.7	1.70	1.70

Section 11.4.8 Exception:

2. Structures on Site Class D sites with S1 greater than or equal to 0.2, provided the value of the seismic response coefficient Cs is determined by Eq. (12.8-2) for values of $T \le 1.5T_s$ and taken as equal to 1.5 times the value computed in accordance with either Eq. (12.8-3) for $T_L \ge T > 1.5T_s$ or Eq. (12.8-4) for $T > T_L$.

Values for Section 11.4.8 Exception (as required):

for	$T \le 1.5\,T_s$	C_s = 0.6213 -	<--Use
for	$1.5\,T_s < T \le T_L$	C_s = 3.8188 -	
for	$T > T_L$	C_s = 355.38 -	
		$C_{s,use}$ = 0.6213 -	

$C_{s,use}$ = 0.6213 - Seismic Response Coefficient

			Weight Overrides							
#	Δh (ft)	Width (ft)	W (lb)	w (plf)	w (psf)	h (ft)	z (ft)	W (lb)	F_u (kip)	M_u (k-ft)
1	0.3	2.3				0.3	0.1667	11.25	0.007	0.0012
2	11.7	2.4				12	6.1667	422.92	0.2628	1.6204
3						12	12	0	0	0
4						12	12	0	0	0
5						12	12	0	0	0
6						12	12	0	0	0
7						12	12	0	0	0
8						12	12	0	0	0
9						12	12	0	0	0
10						12	12	0	0	0
11						12	12	0	0	0
12						12	12	0	0	0
13						12	12	0	0	0
14						12	12	0	0	0
15						12	12	0	0	0
16						12	12	0	0	0
17						12	12	0	0	0
18						12	12	0	0	0
19						12	12	0	0	0
20						12	12	0	0	0
H =	12 ft	Overall height check								

Seismic Load (per pole):

Base Bending Moment: 1.6 k-ft
Base Shear: 0.2698 kip
WIND GOVERNS

Overstrength (per pole): Ω = 1.75 -

Base Bending Moment: 2.8 k-ft
Base Shear: 0.4721 kip
WIND GOVERNS

Post-Installed Anchors (0.75) per 17.2.3.4.4:

(only when applicable)

Base Bending Moment: 3.8 k-ft
Base Shear: 0.6 kip
WIND GOVERNS

Project #: 2405059

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REVERENCE ENGINEERING

NEW : EXISTING COMPARISON

LOAD	V (kip)	M (k-ft)	T (k-ft)
NEW	0.63	4.15	0.30
EXISTING	1.31	13.51	1.59
CHANGE	-52.1%	-69.3%	-81.0%

MEMBER SELECTION STAGE: 1 LRFD DESIGN

Section Type: SquareHSS

M_u = 4.15 k-ft T_u = 0.30 k-ft φ = 0.9 F_y = 50 ksi
V_u = 0.63 kip L = 12 ft = 144 in Z_{req} = 1.1068 in^3

		Section Name	Z	φM_n			φV_n			φT_n			Combined	
( )	LEAST Z (Flexure Only)	HSS2-1/4X2-1/4X1/4	(in^3)	(k-ft)	D/C	Check	(kip)	D/C	Check	(k-ft)	D/C	Check	D/C	Check
			1.28	4.8	0.8647	OK!	19.502	0.0322	OK!	4.1557	0.0729	OK!	0.8647	OK!
(X)	MANUAL SELECTION	HSS3-1/2X3-1/2X1/8	Z (in^3)	φM_n (k-ft)	D/C	Check	φV_n (kip)	D/C	Check	φT_n (k-ft)	D/C	Check	Combined D/C	Check
			1.93	7.2375	0.5735	OK!	19.732	0.0318	OK!	5.9641	0.0508	OK!	0.5735	OK!

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